

Operation and Maintenance Manual for SHOLAYAR DAMS State of Kerala

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Chief Engineer (Civil DRIP & Dam Safety) Kerala State Electricity Board





Operation and Maintenance Manual for Sholayar Dams

Prepared by the Dam Safety Organisation Kerala State Electricity Board Ltd

(A Government of Kerala undertaking) State of Kerala



Front Cover Photograph: Upstream view of Sholayar Flanking and Sholayar Main dams.

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Government of Kerala

Operation and Maintenance Manual

Sholayar Dams





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Kerala State Electricity Board Ltd Pallom, Kottayam.

November 2019

Kerala State Electricity Board Ltd Dam Safety Organisation

Disclaimer

This Operation and Maintenance Manual for Sholayar Dams in no way restricts the dam operators in digressing from her/his responsibilities. The Dam Operators must exercise appropriate discretion and good judgement based on actual site condition when implementing and using the operation and maintenance manual for managing the workings of the dams and appurtenant structures.

The manual was developed for the purpose of organising and managing the operation, inspection and maintenance of the dams for reducing risk and optimizing performance of the dams as a general guide.

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Message

India has more than 5200 large dams. Their health and safety are of paramount importance for sustainable use of the valuable assets, besides providing protection to the people and property in the downstream areas. The Ministry of Water Resources, River Development & Ganga Rejuvenation through the Central Water Commission (CWC), with financial assistance from the World Bank, started the Dam Rehabilitation and Improvement Project (DRIP) to rehabilitate 198 large dam projects in seven states. Kerala State Electricity Board Ltd, through Government of Kerala participated in DRIP to rehabilitate 37 dams under 12 Hydro Electric Projects in the state.

For managing a dam in a sustainable and scientific manner, it is very crucial for each dam owner to have dam specific Operation and Maintenance Manual that lays down procedures for the daily upkeep of the dam. An Operation and Maintenance Manual for a dam is essential for ensuring its safe functioning and for deriving continued benefits. This Operation and Maintenance Manual for Dam has been prepared following the Guidelines for Preparation, Operation and Maintenance Manuals published by CWC in January 2018 under DRIP and covers requirements for project Operation, Inspection, Maintenance, Instrumentation and Monitoring the health of Dam both during monsoon and nonmonsoon periods.

I recommend the dam officials to use this manual for the efficient and safe Operation and Maintenance of the Dams on regular basis.

I compliment all the experts who have contributed to the development of this manual and congratulate the Dam Safety Organisation, KSEB Ltd, Pallom and CWC for the initiation of such important policy protocol to address dam safety management in Kerala.

Bibin Joseph,

Director Generation (Civil), KSEBLtd Kerala

Foreword

Globally, the Operation and Maintenance (O&M) Manual of a dam is one of the most important documents which is supposed to be put in practice right from the initial filling of reservoirs. In order to address the operation and maintenance aspects, ongoing Dam Rehabilitation and Improvement Project (DRIP) has requisite scope to prepare new or update existing O&M manuals for all DRIP dams, which will become very helpful to Dam Owners in addressing the dam specific issues comprehensively in future.

This Operation and Maintenance (O & M) Manual developed is a detailed set of written descriptions with step-by-step procedures for ensuring that the dam is safely operated, frequently inspected and properly maintained. In this era of shrinking budgets, timely inspection and preventative maintenance is necessary for the safe functioning of the dam and continued productive use of the dam and reservoir.

The format of this manual is prepared following the principles published in 2018 CWC Guidelines for Operation and Maintenance of dams for the use by all Dam Owners in developing their own site-specific manuals. Each section of the document provides the necessary instructions to operate inspect and maintain their dams.

It is recommended that all dam officials in charge to use this manual for ensuring that the dam is operated and maintained in a sustainable manner and will continue to derive benefits.

Smt. Supriya S, Chief Engineer (Civil – Dam Safety & DRIP) Kerala State Electricity Board Ltd, Pallom, Kottayam

PREFACE

Operation and Maintenance Manual is a detailed written document of procedures and protocols for ensuring that a Dam is operated and maintained properly and timely to avoid further health deterioration and extend service life of these assets. An Operation and Maintenance Manual is essential for a Dam for ensuring its safe functioning and for deriving desired benefits from it by describing all the elements systematically for its operation, inspection, maintenance, instrumentation and monitoring of the health.

Central Water Commission has published the guidelines for the development of new manual and updating of existing manual vide CDSO_GUD_DS_03_v1.0 Page xii January 2018. Accordingly Kerala State Electricity Board Ltd is developing and updating the Operation and Maintenance Manual of Dams under their ownership for a healthy Dam Safety management system.

Sholayar Dams under KSEBL has an Operation and Maintenance Manual which is not so comprehensive according to the present standards. Hence an attempt is being made here to revise the manual as per the new guidelines by CWC.

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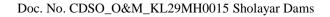
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Name of the Project: Sholayar Hydro Electric Project

Sholayar Hydro Electric Project is the second major hydro electric project commissioned in Chalakudy River Basin in Kerala. The installed capacity of the project is 54 MW (3X18). Annual energy generation expected from this project is 233 Million Units. The Project was commissioned in 1965. The project is located in Athirappilly Panchayath of Thrissur District. The nearest Railway station is Chalakudy (69 km). Nearest Air Port is Cochin International Airport, Nedumbassery, Kochi.

There are three dams as part of this project. They are Sholayar main, Sholayar Flanking and Sholayar saddle dams. These dams create a single reservoir. The spillway arrangement is provided in the flanking dam. The storage capacity of the reservoir is 153.48 Million Cubic Meter. The FRL is 811.68m.



CHAPTER 1 GENERAL INFORMATION

1.1 Introduction

This document represents a detailed Operation and Maintenance (O&M) Manual for Flanking Dam, Main Dam and Saddle Dam of Sholayar Hydro Electric Project, Kerala, providing written descriptions of procedures for ensuring that the dam operates safely and is kept in a good condition by periodic inspections, repairs and maintenance in a sustainable manner. Timely maintenance is important for the continued safe functioning and productive use of the dam and reservoir.

The Manual has been prepared primarily for the dam operation staff and staff supervisors who are assigned the responsibility of physical operation and maintenance of the dam. It contains, as a minimum, all information and instructions necessary for them to perform their allotted tasks in a safe manner. In addition to instructions for dam operation staff, the Manual includes all necessary instructions for other staff directly or indirectly involved in operating and maintaining the dam.

It is essential that the Manual or a copy of the Manual along with supporting data including the atlas of all drawings and manufacturer's technical documents is available at site for ready reference.

1.2 Purpose, Location & Description of Sholayar Hydro Electric Project

The Sholayar H.E. Project is the second Hydel scheme in the Chalakudy basin; the first project being Poringalkuthu left bank scheme. The project comprises of a Main dam across the Sholayar river, a Flanking dam across the valley on the left bank of the river and a saddle dam across the Saddle at Ambalapara. The reservoir formed by these three dams has a capacity of 153.48 Million Cubic Meter. Water from the reservoir is diverted through a tunnel of 738.53 m long to a system of three penstocks of about 822.96 m each. These three penstock pipes lead the water to three turbines of 31100 HP each installed at Anakkayam valley for power generation. The average head on the turbine is 320.04 m.

The project is situated at about $10^{0} 19^{\circ}$ North latitude and $76^{\circ} 44^{\circ}$ East longitude. The project is accessible from Pollachi, a town in Coimbatore District and from Chalakudy the nearest railway station on the Shornur – Cochin line of Southern Railways. The Anamalai Road starting from Chalakudy passes very near the Dam site and Power Station. The distance from Chalakudy to take-off points of road to the Power Station and dam sites are 59.53 km and 64.36 km respectively.



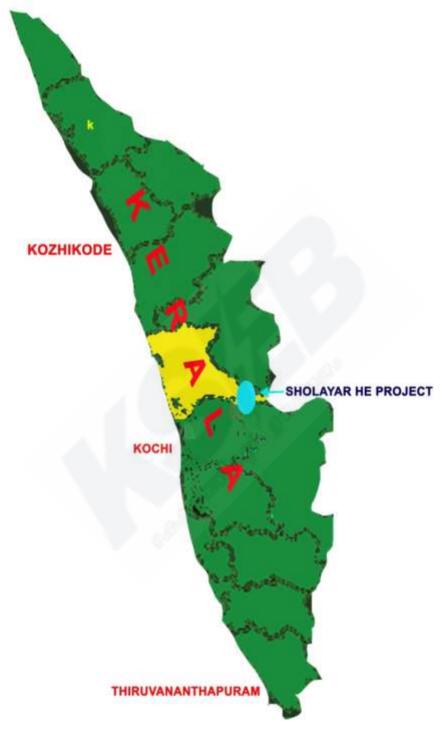


Fig 1.1 Location Map

Location Sketch



Fig 1.2 General Layout of Sholayar Hydro Electric Project



Fig 1.3 Sholayar Hydro-electric Project, view from Google earth.

1.3 Background Details of the Project

The development of Chalakudy river basin was commenced by the erstwhile Cochin State before integration with Travancore and the Poringalkuthu Left Bank Project first stage with 24000 KW installed plant capacity to generate 16000 KW at 50% load factor with one of the three 8000 kw plants as stand bye, was taken up for construction in the year 1946 with Mr. S. J Bruford as the Chief Engineer, H. E Department and completed in 1957.

After integration of the erstwhile Travancore and Cochin States, it was in 1951, after detailed study of the topographical maps of the region, the immense possibilities of very economical power development by diversion of the Sholayar Waters into the head reach of Anakayam Valley was noticed by Sri. V. Ranganathan, then Executive Engineer, Dams Division, Mattupatti (later became Chief Engineer, Civil, KSE Board). As the outcome of the studies conducted by Sri. Ranganathan, a preliminary report on Hydro Electric Development of the Chalakudy River valley was drawn up by him in Sept. 1951 and submitted to the Government.

In his report he had recommended 6 H.E. Schemes to be taken up for detailed study in the basin. The Sholayar H.E. Scheme was suggested as the most economical and most attractive scheme.

There were several alternative ways of exploiting the Sholayar waters for Hydro Electric Generation. Quite an economic scheme was to divert the Sholayar waters south from a storage reservoir in the river down to the Edamalai river through a composite system of tunnel and penstocks. Edamalai is a low level river running west to east roughly parallel to Sholayar joining Periyar above Boothathankettu, north of Kothamangalam. The Power House was to be located at about RL +500.00 at the right bank of Edamalai river. This would have been a high head scheme with a gross head of 609.60 m and over and capable of generating 60000 KW of basic power. The execution of the Poringalkuthu Left Bank Scheme first stage at high cost, the high head scheme referred to above would have been the obvious choice on economic grounds. But as a scheme had already been launched at Poringalkuthu, the proposal of the new scheme and economy of the new scheme must have been considered with what had been done already and so a medium head scheme, a more attractive and more economical scheme utilizing the waters of Sholayar for Power generation and irrigation in the same basin itself was recommended.

The cost per KW of installed capacity of the scheme was then considered to be the lowest in the execution of H.E. Schemes in India. Besides, the scheme had the merit of

making available of a continuous firm draft of 12.74 cumecs for the generation of power in the Poringalkuthu Power Station, the reservoir of which was still lower down in the valley.

The preliminary investigations were commenced from the year 1954-55 and detailed investigations and preliminary works of the scheme began from 1956-57.

The details of the different engineering structures of the project are:

- 1. A main dam across Sholayar river
- 2. A flanking dam across the valley on the left bank of the river.
- 3. A saddle dam across the saddle at Ambalapara.
- 4. A high pressure concrete lined tunnel 710.49 m long, having horse shoe shape and a sectional area of 7.90sqm up to the surge shaft.
- 5. A low pressure pipe line 71.32 m long, 3.20 m dia. from surge shaft.
- 6. A set of 3 surface penstock pipes each about 818.39 m long.
- 7. A Power House at the right bank of the Anakayam stream, a tributary of Chalakudy river accommodating 3 vertical Francis type turbines each of 31000 HP coupled to 3 generators of 18000 KW capacity each.

1.4 Salient Features of the Project

	General		
	River basin : Chalakudy		
		Sub River basin : Sholayar	
		District : Thrissur,	
1	Location	Panchayat : Athirappally	
		Latitude 10°19' N	
		Longitude 76°44' E	
	The nearest Railway station is Chalakudy (69 km).		
2	Means of Access	Nearest Air Port is Cochin International Airport,	
		Nedumbassery, Kochi.	
		Geophysical Features	
1	Catchment area	66.18 km ²	
2	Nature of catchment	The Uppermost reaches are in hilly and forest area	
3	Climate	Moderate {tending to cool}	
4	Mean annual precipitation	305 cm	
5	Silt Charge per year	0.01 Mm ³	

6	Geological features at dam site	Rock type varied from Granitic gneiss to banded gniess
Тe	echnical Details of reservoir	
1	Gross Storage Capacity	153.48 Mm ³
2	Dead Storage	3.40 Mm ³
3	River Bed Level (El.)	+762.00 m
4	Dead Storage Level at MDDL(El.)	+776.94 m
5	Full Reservoir Level (FRL) (El.)	+811.68 m
6	Maximum Water Level (MWL) (El.)	+811.68 m
7	Maximum area of water spread	8.705 Sq. km
A. Flanking Dam		
1	Deepest Foundation Level (El.)	+785.16 m
2	River Bed Level (El.)	+768.10 m
3	Crest level (El.)	+805.28 m
4	Top Level of Dam	+812.90 m
5	Length of dam	259.08 m
6	Height above deepest foundation	27.74 m
7	Width of Dam at top	4.27 m
8	Type of spillway	Ogee
9	No. of Bays	5
10	Type & size of gates	Radial, 6.40 m (W) X 10.96 m (H)
11	Spillway capacity	1739 m3/s
12	Energy Dissipation arrangement	Ski - jump
		B. Main Dam
1	Top Level of Dam	+812.60 m
2	Deepest Foundation Level (El.)	+746.45 m
3	Outlet level	+768.09 m
4	Details of outlet works	1 No. disperser valve 1.8 m dia.
5	Emergency gate at face of outlet	Size 3.10m (W) X 1.63m (H)
6	Type of gate hoist system	Rope drum Suspension beam for the gate hoist moves along with the gate, so that the gate is supported by 2 ropes till it reaches the bottom near the opening. Beyond this, the suspension beam is held in position and from there the gate is suspended by 4 ropes.
7	Length of dam at top	430.53 m

8	Width of dam at top	7.32 m
9	Width at deepest foundation	55.78 m
10	Height above deepest foundation	66.15 m
		C. Saddle Dam
1	Top Level of Dam	+813.82 m
2	Deepest Foundation Level of masonry portion (El.)	+795.22 m
3	Length of dam at top	109.12 m
4	Width of dam at top(masonry portion)	1.83 m
5	Width of dam at top(earth embankment)	6.10 m
6	Height above deepest foundation of masonry portion	18.59 m
	·	Power Tunnel
	Length	710.49 m
	Shape	horse shoe
	Sectional area	7.90sqm
	Maximum flow	23.36 cumecs
	Designed velocity of flow	2.96 m/second
	Inlet level at intake	+718.46 m
	Slope	1 in 120
		Surge Shaft
	Type Circular 9.14 m dia. at the bottom up to elevation +809.5 and 15.24 m dia. up to elevation +816.86 m	
	Length of surge gallery	60.96 m
	Size of surge gallery	3 m x 3 m rectangle surmounted by a semi-circle at one end and 3 m x 1.5 m rectangle surmounted by a semi-circle at the other end
		Power House
	Size	50.29 m x 14.02 m x 16.76 m
	Generator floor (El.)	+475.48 m
	Type of turbine	3 Vertical Francis type turbine each of 31000 HP
	Generators	11 KV, 3 Nos. of 18000 KW each
	Maximum head on turbines	339.24 m
	Overhead travelling crane capacity	75 Tonnes

Table 1.1 Salient Features of Sholayar Hydro Electric Project



Fig 1.4 Upstream view of the Flanking Dam



Fig 1.5 View of Main Dam



Fig 1.6 View of Saddle Dam

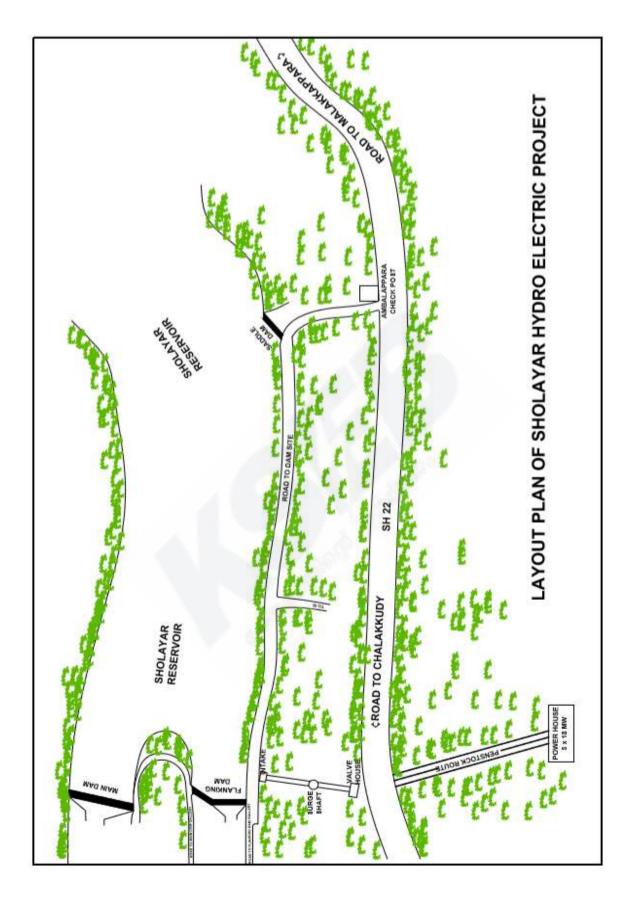


Fig 1.7 Layout Plan

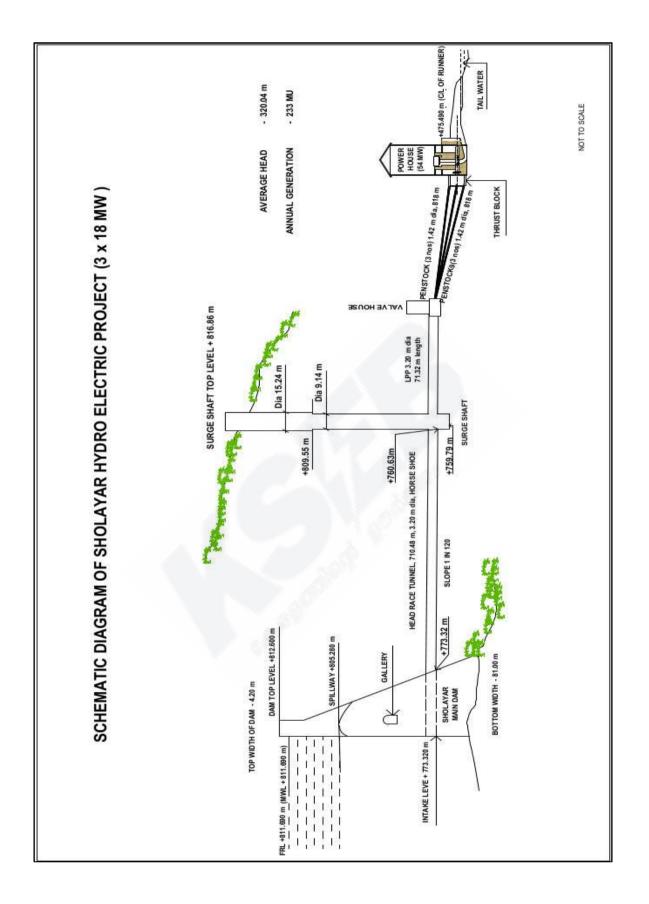


Fig 1.8 Schematic Diagram

1.5 Assignment of Responsibility

Kerala State Electricity Board Ltd is the owner and has the final authority and responsibility for the operation and maintenance of the dams. Identification of all areas of responsibilities connected with the operation and maintenance of the dam are covered in this section. The officer's responsibilities for the various functions (civil, mechanical, electrical, instrumentation etc.) are identified by their designation and, in particular, the responsibilities of operating personnel are specifically identified below, including the regularly scheduled duties, which the staff personnel are required to perform. The same are outlined in the following tables.

Sl No	Particulars	Remarks
1	Project Administration Officer	Chairman & Managing Director, KSEBLtd.
2	Chief Controlling Officer	Chief Engineer (Civil – DS&DRIP).
3	Authority of operation of Spillway and Flood releases	Chief Engineer (Civil – DS&DRIP),KSEBLtd
4	Operation and safety of the dam	Deputy Chief Engineer, Research & Dam Safety Organization& DRIP, KSEBLtd., Pallom
5	Controlling / Operation Officer at dam site	Executive Engineer, Research & Dam Safety Division No. III, Idamalayar.
6	Dam Health Engineer	Executive Engineer, Research & Dam Safety Division No. III, Idamalayar.
7	Recording reservoir data, inspection, maintenance	Assistant Executive Engineer, Dam Safety Sub Division, Poringalkuthu

Table 1.2 – Overall Responsibilities for Sholayar Dams.

1.5.1 Roles and Responsibilities of the AEE and AE during Monsoon

	Flood condition assessment, warning, flood mitigation, and other responsibilities
1	Coordinate with the Project Engineers of Upper Sholayar Project of Tamil Nadu to get the information on the rainfall in the catchment and inflow status and to bring it to the notice of the EE/Dy CE.
2	Assist the EE/ Dy CE /CE to issue notification to the inhabitants downstream in Newspapers, Radio, TV News channel to be alert regarding the flood situation.
3	Assist the EE/ Dy CE /CE to coordinate with the Revenue authorities (District Administration) to alert the downstream inhabitants to evacuate the flood zone to prevent loss of life and livestock.
4	Assist the EE/ Dy CE /CE to coordinate with the CWC flood monitoring authorities on the flood condition
5	Maintain the reservoir water level gauge register and to update on hourly basis during floods and report to EE/ Dy CE /Chief Engineer
6	Assess the inflows in the reservoir as per the approved reservoir operation and to prepare proforma consisting of the status of the reservoir capacity and releases from the reservoir as per the standard Performa and to submit to the EE/ Dy CE /CE
7	Submit to the EE/ Dy CE /CE on the inflows and releases from the reservoir and status of the reservoir twice in the day
8	Maintain the spillway crest gate operation log book
9	Operate the Spillway crest gates for flood mitigation as per the instructions of the EE/ Dy CE /CE and to update the Gate operation Log book
10	Observe the seepages in the drainage Gallery with respect to the reservoir head and record the seepages in the infiltration gallery and to immediately bring to the notice of the EE/ Dy CE /CE in case of excessive seepage/leakage in any specific blocks and porous drains.
11	Observe the gates and to see that its drain holes are not clogged and floating debris is not deposited in the gate components
12	Monitor the condition of the Welding transformers, gas cutting sets, umbrellas, tool kits torches chain blocks ropes bellies etc. on daily basis and to see that things are in

	place to handle any emergency situation
13	Observe the Gates, hoists and handling equipment during operation for the smooth movements and to immediately report any untoward excessive sounds in the motors, pumps or vibrations in the gate
14	Observe the dam top, embankment, catwalk, approach roads are well maintained by housekeeping personnel
15	Observe the performance of the Dam and its appurtenant structures / Gates and Hoists during flood water releases and to report to the EE/ Dy CE /CE in case of any untoward incidents or malfunctioning of the gates of excessive seepages, leakages etc.
16	Assist EE/Dy CE /CE to coordinate with the Generating staff of Sholayar Power House downstream in the operation and power generation.
17	Assist EE/Dy CE /CE to share the flow data and the reservoir storage details to the Media on day to day basis during flood.
18	Assist EE/ Dy CE /CE to coordinate with the Project state of Tamil Nadu and sharing the details of the flood condition in the river and reservoir releases

Table 1.3 – Roles & Responsibilities of AEE & AE

1.5.2 Roles and Responsibilities of the DYCE and EE during Monsoon

SL No	Flood condition assessment, warning, flood mitigation, and other responsibilities
1	Conduct Periodical inspections to assess the health of the Dam and to direct the Executive Engineer for the immediate repair and maintenance for the smooth operation.
2	Observe the performance of the Dam and its appurtenant structures / Gates and Hoists before and after monsoon and to issue necessary instructions to the Executive Engineer
3	Coordinate with the Project Engineers of the State of Tamilnadu & to get the information in the rainfall in respect of Upper Sholayar catchment and inflow status at the state boundary and to bring it to the notice of the CE
4	To issue notification to the inhabitants downstream in Newspapers, Radio, TV News channel to be alert regarding the flood situation

5	Assist the CE to coordinate with the Revenue authorities (District Administration) to alert the downstream villagers to evacuate the flood zone to prevent loss of life and live stock	
6	Assist the CE to coordinate with the CWC flood monitoring authorities on the flood condition	
7	Submit to the CE the daily inflows and releases from the reservoir and status.	
8	Operate the Spillway crest gates for flood mitigation as per the instructions of the CE and to update the Gate operation Log book	
9	Observe the seepages in the drainage Gallery with respect to the reservoir head and record the seepages in the infiltration gallery and to immediately bring to the notice of the CE in case of excessive seepage, leakage in any specific blocks and porous drains	
10	Observe the Gates, hoists and handling equipment during operation for the smooth movements and to immediately report any untoward excessive sounds in the motors, pumps or vibrations in the gate	
11	Observe the dam top, embankment, catwalk, approach roads are well maintained by housekeeping personnel	
12	Observe the performance of the Dam and its appurtenant structures / Gates and Hoists during flood water releases and to report to the CE in case of any untoward incidents or malfunctioning of the gates of excessive seepages, leakages etc.	

Table 1.4 – Roles & Responsibilities of Dy. CE& EE

1.5.3 Roles and Responsibilities of the Chief Engineer during Monsoon

SL No	Flood condition assessment, warning, flood mitigation, and other responsibilities
1	To issue sanction for flood release notification after discussing with Kerala Disaster Management Authority and Revenue Authority (District Administration).
2	Coordinate with the CWC flood monitoring authorities on the flood condition
3	Issue necessary instructions to the Engineers to Operate the reservoir based on the in-flows, rainfall data, releases from the upstream reservoirs and status of the reservoir
4	Observe the performance of the Dam and its appurtenant structures / Gates and Hoists during flood water releases and to issue necessary instructions to the DyCE/EE

5	Coordinate with the Generation wing of KSEBL regarding the power generation requirement.
6	Conduct Pre and Post Monsoon inspections of the Dam and submit the report to CWC.

Table 1.5 - Roles & Responsibilities of the Chief Engineer

1.6 Collection & Reporting of Dam and Reservoir Data

Dam Reservoir Data and vital information as below are collected, recorded and documented for the record.

- Reservoir water surface elevation.
- Reservoir inflow.
- Spillway outflow.
- River releases.
- Hydropower releases.
- Weather related data
- Instrumentation data
- Water quality

MWL (m)	FRL (m)	Crest Level (m)	Present Water Level (m)	Previous Year Water Level	Percentage Storage	Rainfall (mm)	Generation (Mu)	Spill	Gate operation details
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Table 1.6 Daily Reservoir Data

Instruction is given to the Executive Engineer for daily collection and reporting of inflow and outflow data in standard proforma as in **Table 1.6** above to the Deputy Chief Engineer.

On collecting the details in the above format, a daily reservoir status is submitted to the Chief Engineer as in the **Table 1.7**.

Date	Water Level	Previous Year Same day Water Level	Rainfall	Previous Year Rainfall	Storage	Generation	Gross Inflow	PH Discharge + Losses	Spill	Net Inflow	Remarks	
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Table 1.7 Daily Reservoir Status

Records/Logbooks of the operations at Sholayar Dam are maintained in a chronological manner for reference. These records are helpful for identifying preventative maintenance measures that may need to be taken up, troubleshooting the cause of potential equipment failure and documenting development of any unusual conditions.

- Date and Time
- Attendance statement during normal operations both during monsoon and nonmonsoon periods.
- Operations of the spillway gates and outlet works.
- Operating hours of mechanical equipment.
- Testing / Operation of spillway gates, and associated controls.
- Maintenance activities carried out.
- Reservoir and dam inspections.
- Unusual conditions or occurrences.
- Safety and special instructions.
- Names of officers and staff carrying out inspections and maintenance.

Periodical collection of Dam and Reservoir Data are done as follows:

Reservoir water surface elevation:	This is collected daily		
Reservoir inflow:	This is calculated daily		
Spillway outflow:	This is calculated during spill		
River releases:	The tail water release is measured at power		
	house		
Hydropower releases:	The reservoir water is used for power		
	generation		

Weather related data:	Collected and reported daily
Security arrangements:	Provided at three security check posts near
	Dam. CCTV surveillance will be provided
	soon covering the dam and premises.
Water quality:	The quality of water is tested at Regional
	analytical laboratory, Kakkanad, Ernakulam
	district.
Attendance statement during normal	Both during monsoon and non-monsoon
operations	period maintained at field office
Operations of the spillway gates and	Take record of actual operations
outlet works	
Operating hours of mechanical	Maintained at field office
equipments	
Testing/Operation of spillway gates	The testing and operation are being carried
and associated controls	out as per the manual and maintenance
	schedule. Other details maintained at field
	office
Testing/operation of Outlet gates,	Maintained at field office
valves and associated controls	
Maintenance activities carried out	Details maintained at field office
Reservoir and dam inspections	Periodically inspected and details maintained
	at field office
Unusual conditions or occurrences,	Details maintained at field office
including acts of vandalism	
Attendance statement at dam during	Details maintained at field office
emergency operations	
Changes to normal operating procedure	Details maintained at field office
Communication network checks:	Regularly checked and maintained
Safety and special instructions:	Safety equipment provided
Names and addresses of official visitors:	Record of inspections maintained at office.

1.7 Public and Project Staff - Health and Safety

As safety of Project Staff is of prime concern, safety instructions & protection measures at the dam are to be followed by all staff / project personnel. Security personnel are located on the left abutment for providing public notices of events and status of security of the dam and downstream river conditions.

Access Roads

The Project is accessible from Pollachi, a town in Coimbatore District of Tamil Nadu State and from Chalakudy the nearest railway station on the Shornur – Cochin line of Southern Railways. The Anamalai Road starting from Chalakudy passes very near the Dam site and Power Station. The distance from Chalakudi to take-off points of road to the Power Station and dam sites are 59.53 km and 64.36 km respectively.

The dam site was made accessible through an approach road of about 1.80 km long branching off from Chalakudi Anamalai road. The Power House site was also made accessible through an approach road of about 1.89 km. Branch roads to the temporary staff colony, semi-permanent staff colony and permanent staff colony were also constructed from the approach road to dam site. Altogether about 13.50 km of new access roads were constructed for this scheme and are maintained by KSEBL.

Location of public utilities / conveniences

Inspection Bungalow and Canteen are located near the dam. Police station is located at Athirappally, 45 km away from the dam. A Govt. Primary Health Centre is located at Nurseryppadi near Athirappally, 55 km away from dam.

Safety equipment available at the dam: First aid kit and fire extinguishers are available.

1.8 Restricted Areas

Certain areas of the dam and reservoir are restricted for entry of the general public. The purpose of restrictions is for security of the dam, public safety and uninterrupted safe operation of the dam. Restricted areas will include the following:

- Confined spaces such as adits, galleries etc.
- Spillway approach areas, chutes and energy dissipation arrangements.
- Intake structure of the tunnel.

Sign boards are displayed at the prohibited areas of the dam.

1.8.1 Details of the Security arrangements at Sholayar Dam Site

Security arrangements are already provided through private agency at three security check posts near dam. Also CCTV surveillance will be provided soon covering the dam and premises.

Security Arrangement Existing	-	Private agency (Ex – Servicemen)
No of security persons engaged	-	3 persons at a shift.

1.9 Staff Position, Communication & Warning System

The number & description of operating unit personnel posted/placed at different locations of the dam are indicated in supporting documents. An engineering organizational chart for the control and safety of Sholayar dam is shown in **Figure 1.9** below. Means of communications both in normal and emergency situations are identified in the Communication Directory. Communication means available include land line, mobile phone and satellite phones. An organizational structure for the purpose is shown below:

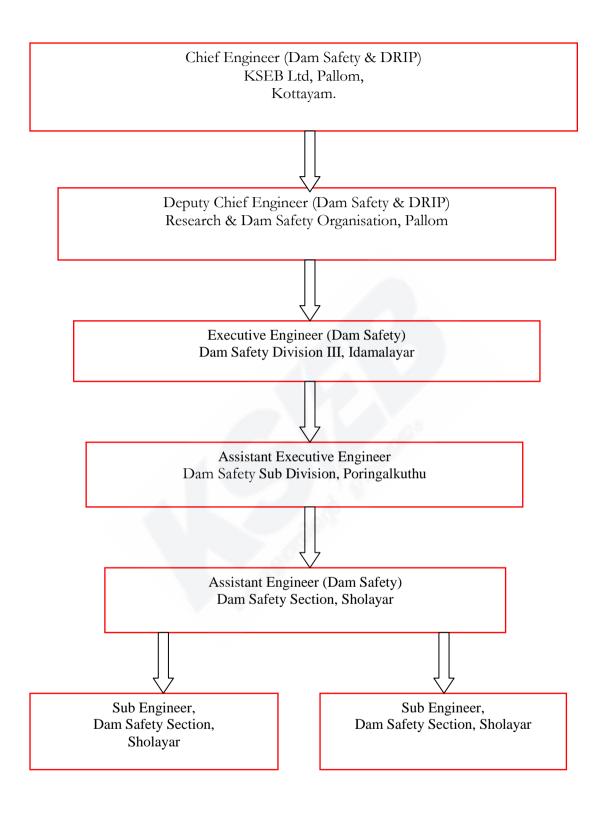


Fig 1.9 – Dam Safety Organisation Structure for Sholayar Dam

Present hierarchy of Controlling officers and their contacts are as below:

Chief Engineer Civil (Dam safety & DRIP), KSEBL, DSO Building, Pallom, Kottayam Phone: 9496018719, e-mail: cedamsafety@gmail.com

Deputy Chief Engineer, Research & Dam Safety Organization, Pallom Phone: 9446008492, 0481-2432290, 9496011540 e-mail: dirroplm2@gmail.com.

Executive Engineer, Dam Safety Division No. III, Idamalayar Phone: 9446008426, e-mail: <u>ddidamalayar@gmail.com</u>.

Assistant Executive Engineer, Dam Safety Sub Division, Poringalkuth Phone: 9496018372, e-mail: <u>aeedrippgl@gail.com</u>

Assistant Engineer, Dam Safety Section, Sholayar. Phone: 94960119979

Spillway flood releases

As per the guidelines issued by the Central Water Commission for developing Emergency Action Plan for Dams issued in February, 2016 (Doc. No. CDSO_GUD_DS_01_v2.0), a three colour alert system (Blue, Orange and Red) shall be issued by the dam owner on detecting a distress in the dam according to the nature of the emergency level, to the various statutory authorities in charge of District Administration, Disaster Management etc. in order to take appropriate action to reduce the risk to lives and property from the consequences of potential dam failures. The above guidelines also envisage using the alerts in the case of a large controlled release from the dam due to severe weather and emergency conditions. A three stage/colour alert system is issued for the major reservoirs of KSEBL based on the above for a controlled release through the spillway for enabling the District Administration to take disaster mitigation measures.

In Sholayar reservoir, the first warning (Blue Alert) as water level reaches 1.50m below the rule level, second warning (Orange Alert) as water level reaches 1m below the rule level and third warning (Red Alert) as water level reaches 0.50m below the rule level are given for opening of spillway gates. Warnings are given in local media including TV etc. regarding the possible opening of spillway gates continuously up to rule level. Also intimations are given to Disaster Management, District Administration, and Police

Department etc. Spillway gates are opened at rule level based on 'Guidelines for Operation of Reservoirs' (IS 7323:1994) and Gate Operation Manual. As per the PAP Agreement the water level has to be maintained at FRL of 811.68 m on September 1st and February 1st of every water year. During the period from September 2nd and January 31st the water level has to be kept between 810.16 m and FRL.

The different stage/colour alert levels are fixed considering the normal rainfall intensity and for general guidance. In the case of extreme rainfall event or identifying any distress in the dam, appropriate protective action shall be initiated by the Dam managers. Sanction has to be obtained from the District Collector for spilling the water. Evacuation is required at plains in the case of large release/extreme rainfall event.

Release for hydropower

Water from the reservoir is mainly used for power generation at **3 x18 MW** power house of KSEBL. The tail water from Sholayar power house is collected in Poringalkuthu reservoir and is used for power generation at Poringalkuthu Power house. There is no water supply arrangement directly from the reservoir. The entire water after generation at an average 349.50MCM per annum is released to downstream from the reservoir.

Routine inspection

Usually monthly inspection and quarterly inspections as per KDSA are carried out by the operating/controlling officers. Pre-monsoon inspection and Post-monsoon inspection as per CWC are carried out and reports are supplied to CWC. The pre-monsoon and postmonsoon reports are to be updated in DHARMA web site.

Maintenance

Routine maintenance is carried out for Spillway gates, Intake gates as part of routine maintenance before the onset of monsoon. Details are given under the Chapter, Project Maintenance.

1.10 Typical Schedule of Duties

Schedule of duties/inspections to be carried out for the operation and maintenance of the dam by the concerned official are tabulated below:

Sl. No.	Component/ Duty	Frequency	Personnel
1	Visual inspection of dam including Crest of dam (Dam top), Upstream and downstream faces, visible portions of foundation and abutments, Galleries, Spillway and its energy dissipation arrangements, Power Intake	Daily	Sub Engineer/Dam operators on contract
2	Record water surface elevation, reservoir inflow and spillway discharge.	Daily (Hourly basis during monsoon)	Sub Engineer/Dam operators on contract
3	Record meteorological data, Record releases from outlets /sluices	Daily	Sub Engineer/Dam operators on contract
4	Check security and safety devices, Complete logbook / site register which include the above information.	Daily	Assistant Engineer
5	Record seepage from drainage systems, Gallery drains etc. and record meteorological data.	Weekly	Sub Engineer/Dam operators on contract
6	Visual inspection of dam including Crest of dam (Dam top), Upstream and downstream faces, visible portions of foundation and abutments, Galleries, Spillway and its energy dissipation arrangements, Power Intake	Weekly	Assistant Engineer
7	Check stand by generator (DG Sets), Drainage systems, Gallery drains etc.	Weekly	Assistant Engineer
8	Visual inspection of dam including Crest of dam (Dam top), Upstream and downstream faces, visible portions of foundation and abutments, Galleries, Spillway and its energy dissipation arrangements, Power Intake	Fort nightly	Assistant Executive Engineer

9	Check security and safety devices, logbook and site register which include the above information.	Fort nightly	Assistant Executive Engineer
10	Check stand by generator (DG Sets), Drainage systems, Toe drains, Gallery drains etc.	Fort nightly	Assistant Executive Engineer
11	Measuring devices, communication devices, status of instruments, vegetation growth	Fort nightly	Assistant Executive Engineer
12	Check Sign/Warning display boards near vulnerable locations	Fort nightly	Assistant Executive Engineer
13	Visual inspection of dam including Crest of dam (Dam top), Upstream and downstream faces, visible portions of foundation and abutments, Galleries, Spillway and its energy dissipation arrangements, Power Intake	Monthly	Executive Engineer
14	Check measuring devices/ Instruments, Security and safety devices, Communication Devices, Status of Vegetation growth – rectification, if needed.	Monthly	Executive Engineer
15	Check Sign/Warning display boards near vulnerable locations	Monthly	Executive Engineer
16	Replace fuse light bulbs, Inspect to maintain ventilation system, cleaning of control panel boards.	Monthly	Assistant Engineer
17	Check outlet works, updating operating instruction, check gate air vents, clean gate control switchboxes, check operation of gates, grease gate hanger/dogging	Quarterly	Executive Engineer
18	Check condition of trash rack of intake structure, Check condition of Outlet works & its Energy Dissipation Arrangement, Check operation of Valve house	Quarterly	Executive Engineer
19	Check condition of spillway, log and safety boom, Check for debris in inlet channel, Check operation of gates, Check for damages in spillway glacis, energy dissipation arrangement,	Quarterly	Executive Engineer

r	1		1
	d/s area etc., Check and clear		
	spillway bridge drains, Clean		
	inside of motor control cabinet.		
20	Check for adherence to	Quarterly	Executive Engineer
	instrumentation schedule,		0
	Record pertinent information in		
	Operation of Gates, Check		
	condition of V-notch/seepage		
	measuring devices, Check hydro		
	mechanical components.		
21	Inspection of Spillway & outlet	Half yearly (Pre	Deputy Chief Engineer
	works, hydro mechanical	and Post	along with Executive
	components, Check paint on	Monsoon)	Engineer in charge of
	gates, Check lubrication of wire		dam
	ropes and application of cardium		
	compound, Check mechanical		
	hoist bearings and flexible		
	coupling bearings, Check gear		
	systems, Exercise gates and		
	valves, Check oil reservoir level		
	in hydraulic system, Check		
	pressure release valve, Check		
	lubrication of gate rollers, Check		
	rubber seals and seal clamp bar.		
22	Submission of Inspection report	Half yearly	Chief Engineer/Deputy
	to State DSO, CWC and	finit yearly	Chief Engineer
	uploading into DHARMA.		
23	Comprehensive inspections	Annually	Dam Safety Authority
23	Comprehensive inspections	Tinitiany	
24	Towned the set of the	E' V 1	along with Dam Owners
24	Inspect dam and gate structures,	Five Yearly	Chief Engineer/Deputy
	trash racks and stilling basin /		Chief Engineer
	energy dissipation arrangement,		
	which normally are underwater		
	(by dewatering or by		
	divers/ROV as necessary).		
	Review Dam operation		
	procedures and EAP and update		
	as necessary.		
25	Comprehensive inspection of	Ten Yearly	DSRP
	performance of the dam and		
	gate structures and reservoirs,		
	trash racks and stilling basin/		
	energy dissipation arrangement.		
	intersy anonpation arrangement.		

Table 1.8 Schedule of duties/inspections

1.11 Hydro-Mechanical Inspections / Checks

Special duties required to be performed by H-M operating personnel are given in this section. Frequency of inspections / checks for hydro-mechanical components and necessary actions to be taken up during maintenance are as under:

- 1. Gates
- a. Embedded Parts

Sl. No	Embedded Part	Frequency
1	Checking of seal beams. Seal Seats, Guide track & all other exposed embedded parts with respect to their alignment, distortion : if any due to continuous use, pitting and un-necessary cracks due to wear & carrying out requisite repairs, rectification by welding, grinding etc.	Half Yearly
2	Removing debris & other foreign material deposited on embedded parts& cleaning the same.	Monthly
3	All cracks & defective weld joints to be ascertained & rectified.	Half Yearly
4	All dirt, debris, grit, foreign material etc. to be removed from trunnion assemblies as well as trunnion chair and lubricate trunnion bearing & the sliding surface on trunnion chair with specified lubricant/ grade to ensure smooth sliding movement of trunnion.	Monthly
5	All nut& bolts connecting Trunnion Assembly & Trunnion Chair and Trunnion & Yoke, girder, Trunnion pin lock plate to be checked & Tightened and replacement of the same, if found defective.	Monthly

Gate structure

Sl. No	Gate Parts	Frequency
1	Regular inspection of the gate along with the hoist to be carried out daily to ensure that there is no unusual development/ observation	Daily
2	Check all welding for soundness & rectify defects	Quarterly

3	Check welding between arms & horizontal girders as well as arms & Trunnion with the help of magnifying glass for cracks/ defects and rectify the defects	Quarterly
4	Clean all drain boles including those in end arms, horizontal girders & defective nuts & bolts	Quarterly
5	Check all nuts & bolts provided and tighten them, and replace the defective nuts & bolts	Quarterly
6	Check upstream face of Skin plate for pitting, scaling and corrosion. Scaling formation are to be removed. Pitting shall be filled with weld &Corroded surface shall be cleaned & painted	Yearly
7	Joints of side & bottom rubber seals to be checked for their proper alignment and fixing & to be rectified/ adjusted if there is leakage through joints	Monthly
8	Nuts & bolts for rubber seal connection to be tightened and damaged nuts and bolts to be replaced	Quarterly
9	The excessive or widespread leakages if any shall be reported to the Engineer in charge. If the seals are required to be replaced the same shall be carried out	Quarterly
10	The guide roller pin is to be lubricated	Quarterly

Table 1.9 Frequency of inspection of Hydro Mechanical components

1.12 Supporting Documents & Reference Material

This O&M Manual is the key instruction document. Supporting documents and necessary instructions for all phases of the operation, inspection and maintenance of the dam, reservoir and appurtenant works shown below are available at the dam control room:

- Emergency Action Plan (EAP)
- Flood forecasting and operating criteria
- Basin or river operating plan
- Interstate agreements
- Agreements with other user agencies
- Power station operation plan
- Administrative procedures

- Reservoir / River pollution contingency plan
- Maintenance schedules
- Gate Manufacturer's instructions and drawings
- Regional communication directory
- Instrumentation reports / results

CHAPTER 2

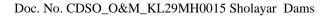
PROJECT OPERATION

2.1 Basic Data

The Sholayar operation plan consists of step-by-step instructions for operating the dam and reservoir during routine (normal) and emergency conditions. The operating procedures for normal operations are discussed in this chapter including operating criteria for the reservoir, spillway and outlets. The operation of a dam involves regulation of its reservoir as per rule curve/ project specific requirements. This includes the use of elevation storage curve and design flood; both these aspects are described below.

2.1.1 Elevation Storage curve

The Elevation - Storage Curve for Sholayar Reservoir in tabular and graphical form are shown in Fig. 2.1 and Table 2.1.



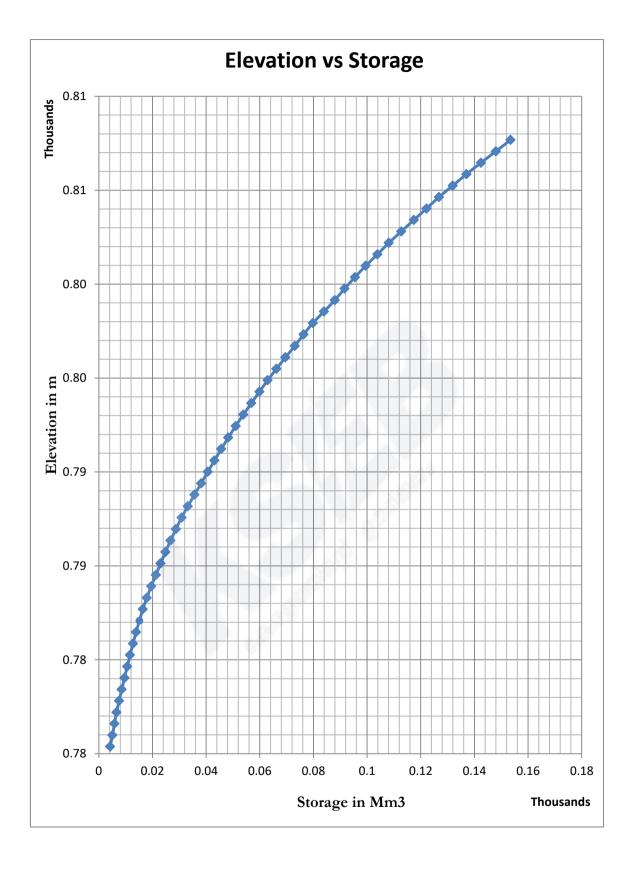


Figure 2.1 - Elevation storage curve for Sholayar Reservoir

MDDL (m)	FRL (m)	MWL (m)		Dead Storage in Mm ³
776.94	811.68	811	.68	3.4
Reserv	voir level	Gross	storage	Remarks
in ft	in m	in Mcft	in Mm ³	
2557.00	779.37	150.32	4.26	
2559.00	779.98	179.40	5.08	
2561.00	780.59	206.00	5.83	
2563.00	781.20	234.00	6.63	
2565.00	781.81	267.00	7.56	
2567.00	782.42	302.00	8.55	
2569.00	783.03	338.00	9.57	
2571.00	783.64	374.00	10.59	
2573.00	784.25	411.00	11.64	
2575.00	784.86	450.00	12.74	
2577.00	785.47	491.00	13.90	
2579.00	786.08	534.00	15.12	
2581.00	786.69	580.00	16.42	
2583.00	787.30	632.00	17.90	
2585.00	787.91	690.00	19.54	
2587.00	788.52	751.00	21.27	
2589.00	789.13	813.00	23.02	
2591.00	789.74	877.00	24.83	
2593.00	790.35	944.00	26.73	
2595.00	790.96	1015.00	28.74	
2597.00	791.57	1090.00	30.87	
2599.00	792.18	1172.00	33.19	
2601.00	792.78	1259.00	35.65	
2603.00	793.39	1347.00	38.14	
2605.00	794.00	1435.00	40.63	
2607.00	794.61	1523.00	43.13	
2609.00	795.22	1612.00	45.65	
2611.00	795.83	1705.00	48.28	

2613.00	796.44	1802.00	51.03	
2615.00	797.05	1903.00	53.89	
2617.00	797.66	2008.00	56.86	
2619.00	798.27	2114.00	59.86	
2621.00	798.88	2223.00	62.95	
2623.00	799.49	2338.00	66.20	
2625.00	800.10	2457.00	69.57	
2627.00	800.71	2580.00	73.06	
2629.00	801.32	2695.50	76.33	
2631.00	801.93	2817.50	79.78	
2633.00	802.54	2964.00	83.93	
2635.00	803.15	3106.00	87.95	
2637.00	803.76	3235.00	91.60	
2639.00	804.37	3372.00	95.48	
2641.00	804.98	3514.00	99.50	
2643.00	805.59	3668.00	103.87	
2645.00	806.20	3817.50	108.10	
2647.00	806.81	3982.00	112.76	
2649.00	807.42	4147.00	117.43	
2651.00	808.02	4313.00	122.13	
2653.00	808.63	4478.00	126.80	
2655.00	809.24	4659.00	131.93	
2657.00	809.85	4841.00	137.08	
2659.00	810.46	5030.00	142.43	
2661.00	811.07	5228.50	148.05	
2663.00	811.68	5420.00	153.48	Full Reservoir Level

2.2 Operation Plan

An effective operation plan and schedule is required for the safe project operation for which the project specific features shall be known. Salient features of the Reservoir are given below.

Salient features

Maximum Water Level (MWL)	-	+811.68 m
Full Reservoir Level (FRL)	-	+811.68 m
Minimum Draw Down Level (MDDL)	-	+776.94 m
Gross storage at FRL	- /	153.48 Mm ³
Gross storage at MWL	-	153.48 Mm ³
Live storage at FRL	-	150.08 Mm^3
Dead storage	- <	3.40 Mm ³
Water spread area at FRL	-	870.50 Ha



Fig 2.2 Downstream view of spillway

2.2.1 Data of the historic floods

As per historical records, the maximum flood observed in Western Ghats was during 1924. The centre of the storm of the 1-day rainfall of 17th July 1924 and 2-day rainstorm of July 16-17 was located at Devikulam in Kerala in which rain fall of 484 mm and 751 mm respectively was recorded. Hydrological studies were carried out for the Sholayar dam considering the rainfall data from 1948 -1953.

The SW monsoon of the year 2018 in the State was similar to that of 1924 Devikulam storm and Kerala experienced an abnormally high rainfall from 1 June 2018 to 19 August 2018 which resulted in severe flooding in 13 out of 14 districts in the State. It is seen that the 2-day and 3-day rainfall depths of 15 -17, August 2018 rainfall in Pamba, Periyar and Bharathapuzha sub-basins are almost comparable to the Devikulam storm of 16-18, July 1924. Out of 758.6 mm rainfall from 1 August 2018 to 19 August 2018, about 414 mm rainfall occurred in just three days viz. 15-17, August 2018, which created severe flooding in the state, while the same during 16-18, July 1924 was 443 mm. The three day rainfall of 15-17August 2018 at Sholayar Dam site was 620 mm.

The spillway shutters were open during 2000, 2005, 2007, 2009, 2011, 2013, 2014, 2017 and 2018 for excess flood moderation. Spill details of the above years are tabulated below in **Table 2.2**.

Date	Water Level	Rainfall Sholayar Dam	Gross Storage	Gross inflow	Spill	Net Inflow
Date	Feet	mm	Mcm	Mcm/day	Mcm/day	Mcm/day
29-Aug-00	811.56	34.0	152.2766	2.0713	0.0995	1.2178
3-Sep-00	811.69	3.0	153.4944	1.8083	0.7411	-0.3115
4-Sep-00	811.65	5.0	153.1829	1.2120	0.3097	-0.5947
6-Sep-00	811.53	16.0	151.9934	0.8898	0.1991	-0.8071
		1.35				

2-Aug-05	810.92	69.0	146.7967	9.0659	1.2087	6.6977
3-Aug-05	811.69	45.0	153.4944	10.1625	8.8896	0.0000
4-Aug-05	811.69	17.0	153.4944	3.5520	2.2439	0.0000
5-Aug-05	811.69	29.0	153.4944	1.9307	0.6194	0.0000
6-Aug-05	811.69	3.0	153.4944	1.7506	0.4424	0.0000
7-Aug-05	811.69	0.0	153.4944	1.5262	0.2212	0.0000
1-Sep-05	811.69	13.0	153.4944	1.0380	0.2610	-0.3115
7-Sep-05	811.69	70.0	153.4944	1.5415	0.2430	0.0000
8-Sep-05	811.69	90.0	153.4944	5.6306	4.3256	0.0000
9-Sep-05	811.69	77.0	153.4944	6.1548	4.8617	0.0000
10-Sep-05	811.69	51.0	153.4944	4.2276	2.9336	0.0000
11-Sep-05	811.69	95.0	153.4944	5.3392	4.0351	0.0000
12-Sep-05	811.69	53.0	153.4944	5.1381	3.8338	0.0000
13-Sep-05	811.69	37.0	153.4944	3.7149	2.4126	0.0000
14-Sep-05	811.69	57.0	153.4944	3.1062	1.8086	0.0000
15-Sep-05	811.69	9.0	153.4944	2.6916	1.4118	0.0000
16-Sep-05	811.69	8.0	153.4944	2.4020	1.1050	0.0000
17-Sep-05	811.69	0.0	153.4944	2.1880	0.8848	0.0000
18-Sep-05	811.69	0.0	153.4944	2.6215	1.3697	0.0000
19-Sep-05	811.69	21.0	153.4944	2.5415	1.2377	0.0000
20-Sep-05	811.69	15.0	153.4944	3.0323	1.7573	0.0000
21-Sep-05	811.69	60.0	153.4944	2.7557	1.4549	0.0000
22-Sep-05	811.69	15.0	153.4944	2.3574	1.0611	0.0000

23-Sep-05	811.69	0.0	153.4944	2.2039	0.9064	0.0000
24-Sep-05	811.69	0.0	153.4944	2.1432	0.8406	0.0000
		Total Spill 2005			50.37	
9-Aug-07	811.69	42.5	153.4944	4.1552	2.453	0.0000
10-Aug-07	811.69	180.0	153.4944	7.4349	5.704	0.0000
11-Aug-07	811.69	32.5	153.4944	4.3347	2.387	0.0`000
27-Aug-07	811.69	20.0	153.4944	2.4092	0.4641	0.0000
28-Aug-07	811.69	27.5	153.4944	2.4714	0.5304	0.0000
30-Aug-07	811.69	25.0	153.4944	2.5601	0.6	0.0000
31-Aug-07	811.69	70.0	153.4944	3.0674	1.1267	0.0000
1-Sep-07	811.69	17.5	153.4944	2.5153	0.5746	0.0000
2-Sep-07	811.69	1.0	153.4944	3.2353	1.3694	0.0000
3-Sep-07	811.69	35.0	153.4944	2.5007	0.6188	0.0000
4-Sep-07	811.69	2.0	153.4944	2.5241	0.6851	0.0000
5-Sep-07	811.69	6.0	153.4944	2.7608	0.9503	0.0000
6-Sep-07	811.69	42.0	153.4944	2.6064	0.6630	0.0000
7-Sep-07	811.69	7.5	153.4944	2.1579	0.2652	0.0000
8-Sep-07	811.69	0.0	153.4944	2.7399	0.7956	0.0000
9-Sep-07	811.69	27.0	153.4944	2.8223	0.8840	0.0000
10-Sep-07	811.69	70.0	153.4944	2.3950	0.6851	0.0000
11-Sep-07	811.69	3.0	153.4944	2.5892	0.6404	0.0000
12-Sep-07	811.69	57.5	153.4944	3.2564	1.3031	0.0000
13-Sep-07	811.69	0.0	153.4944	2.3257	0.3757	0.0000

14-Sep-07	811.69	17.5	153.4944	2.5183	0.6409	0.0000
15-Sep-07	811.69	7.0	153.4944	2.6139	0.6630	0.0000
16-Sep-07	811.69	28.0	153.4944	2.3677	0.4199	0.0000
17-Sep-07	811.69	15.0	153.4944	3.0098	1.0605	0.0000
18-Sep-07	811.69	48.0	153.4944	3.4835	1.5456	0.0000
19-Sep-07	811.69	80.0	153.4944	4.6661	2.7380	0.0000
20-Sep-07	811.69	23.0	153.4944	3.1668	1.6354	0.0000
21-Sep-07	811.69	38.0	153.4944	2.7913	1.5028	0.0000
22-Sep-07	811.69	32.5	153.4944	3.0325	1.7456	0.0000
23-Sep-07	811.69	70.0	153.4944	3.8804	2.3300	0.0000
24-Sep-07	811.69	22.5	153.4944	2.7538	0.8177	0.0000
25-Sep-07	811.69	47.0	153.4944	3.5244	1.5912	0.0000
26-Sep-07	811.69	42.5	153.4944	2.6449	0.7072	0.0000
27-Sep-07	811.69	0.0	153.4944	2.4517	0.6188	0.0000
28-Sep-07	811.69	2.5	153.4944	2.8350	0.9724	0.0000
29-Sep-07	811.69	0.0	153.4944	2.4253	0.5525	0.0000
30-Sep-07	811.69	0.0	153.4944	2.8530	0.9060	0.0000
1-Oct-07	811.69	15.0	153.4944	2.5203	0.5746	0.0000
2-Oct-07	811.69	0.0	153.4944	2.2533	0.3094	0.0000
29-Oct-07	811.62	55.0	152.8572	3.4125	0.8440	0.6372
30-Oct-07	811.69	48.0	153.4944	2.7883	0.8398	0.0000
31-Oct-07	811.69	0.0	153.4944	2.5498	0.7072	0.0000
1-Nov-07	811.69	0.0	153.4944	2.3169	0.3978	0.0000

		47.21				
2-Sep-09	811.69	15.0	153.4944	1.1367	0.685	0.0000
3-Sep-09	811.69	55.0	153.4944	1.3031	0.398	0.0000
4-Sep-09	811.69	113.0	153.4944	1.2510	0.818	0.0000
5-Sep-09	811.69	60.0	153.4944	1.2756	1.277	0.0000
6-Sep-09	811.69	45.0	153.4944	1.2765	2.329	0.0000
7-Sep-09	811.69	10.0	153.4944	1.2805	0.972	0.0000
8-Sep-09	811.69	2.5	153.4944	1.5504	0.331	0.0000
3-Oct-09	811.69	60.0	153.4944	2.5716	1.326	0.0000
4-Oct-09	811.69	15.0	153.4944	2.8633	1.567	0.0000
5-Oct-09	811.69	0.0	153.4944	2.1858	0.894	0.0000
6-Oct-09	811.69	0.0	153.4944	1.7526	0.464	-0.3115
		Total Spill 2009		A COLOR	11.06	
1-Sep-11	811.69	89.0	153.4944	1.8596	2.223	0.0000
2-Sep-11	811.69	100.0	153.4944	0.6293	2.252	-1.2178
3-Sep-11	811.56	120.0	152.2766	2.8141	1.148	1.2178
4-Sep-11	811.69	32.0	153.4944	1.8314	2.077	0.0000
5-Sep-11	811.69	22.0	153.4944	1.9071	1.237	0.0000
6-Sep-11	811.69	9.0	153.4944	1.9188	1.545	0.0000
7-Sep-11	811.69	38.0	153.4944	1.8485	1.192	0.0000
8-Sep-11	811.69	3.0	153.4944	1.8846	0.861	0.0000
9-Sep-11	811.69	15.0	153.4944	1.9211	1.193	0.0000
10-Sep-11	811.69	8.0	153.4944	1.9230	0.641	0.0000

811.69	35.0	153.4944	1.5680	1.954	0.0000
811.69	35.0	153.4944	1.9202	0.398	0.0000
811.69	26.0	153.4944	1.8378	1.565	0.0000
811.69	35.0	153.4944	1.6380	0.994	0.0000
811.69	12.0	153.4944	1.2657	1.325	0.0000
811.69	34.0	153.4944	0.8840	1.740	-0.3115
811.65	10.0	153.1829	1.2557	0.574	0.0000
811.65	0.0	153.1829	1.5202	0.839	0.3115
811.69	2.0	153.4944	1.2581	1.055	0.0000
811.69	35.0	153.4944	1.2315	1.236	0.0000
811.69	12.0	153.4944	1.0122	1.214	0.0000
811.69	9.0	153.4944	0.8309	0.848	0.0000
811.69	3.0	153.4944	0.4241	0.397	-0.3115
	Total Spill 2011			28.51	
811.23	30.0	149.3597	10.6762	5.2613	4.1347
811.69	222.0	153.4944	13.9141	12.8255	0.0000
811.69	100.0	153.4944	11.8952	10.6565	0.0000
811.69	35.0	153.4944	7.3438	6.2692	0.0000
811.69	100.0	153.4944	7.2817	6.3134	0.0000
811.69	80.0	153.4944	4.4139	3.1341	0.0000
811.69	5.0	153.4944	5.0260	3.8852	0.0000
811.69	8.0	153.4944	3.4023	2.1192	0.0000
811.69	10.0	153.4944	3.2253	1.9426	0.0000
	811.69 811.69 811.69 811.69 811.65 811.65 811.69	811.69 35.0 811.69 26.0 811.69 35.0 811.69 35.0 811.69 12.0 811.69 34.0 811.65 10.0 811.65 0.0 811.65 0.0 811.65 0.0 811.69 2.0 811.69 2.0 811.69 35.0 811.69 9.0 811.69 9.0 811.69 3.0 811.69 3.0 811.69 3.0 811.69 30.0 811.69 30.0 811.69 30.0 811.69 30.0 811.69 30.0 811.69 30.0 811.69 30.0 811.69 30.0 811.69 30.0 811.69 30.0 811.69 30.0 811.69 30.0 811.69 30.0 811.69 30.0 811.69 80.0 811.6	811.69 35.0 153.4944 811.69 26.0 153.4944 811.69 35.0 153.4944 811.69 35.0 153.4944 811.69 12.0 153.4944 811.69 34.0 153.4944 811.69 34.0 153.4944 811.69 34.0 153.4944 811.65 10.0 153.1829 811.65 0.0 153.4944 811.69 2.0 153.4944 811.69 35.0 153.4944 811.69 9.0 153.4944 811.69 9.0 153.4944 811.69 3.0 153.4944 811.69 3.0 153.4944 811.69 30.0 149.3597 811.69 30.0 149.3597 811.69 100.0 153.4944 811.69 35.0 153.4944 811.69 35.0 153.4944 811.69 35.0 153.4944 811.69 5.	Image: Constraint of the section of the sec	Image: Constraint of the state of

10-Aug-13	811.69	20.0	153.4944	3.1492	1.9426	0.0000
11-Aug-13	811.69	3.0	153.4944	2.4532	1.2805	0.0000
12-Aug-13	811.69	0.0	153.4944	2.4634	1.3033	0.0000
13-Aug-13	811.69	0.0	153.4944	2.6945	1.4144	0.0000
14-Aug-13	811.69	30.0	153.4944	2.2304	1.0817	0.0000
15-Aug-13	811.69	12.0	153.4944	1.8913	0.7952	0.0000
16-Aug-13	811.69	5.0	153.4944	2.4071	1.1928	0.0000
17-Aug-13	811.69	15.0	153.4944	2.2888	1.1485	0.0000
18-Aug-13	811.69	1.0	153.4944	2.8011	1.8546	0.0000
19-Aug-13	811.69	25.0	153.4944	2.3525	1.0608	0.0000
20-Aug-13	811.69	10.0	153.4944	1.9542	0.6630	0.0000
21-Aug-13	811.69	30.0	153.4944	2.3370	1.0608	0.0000
22-Aug-13	811.69	5.0	153.4944	1.5690	0.5304	0.0000
23-Aug-13	811.69	1.0	153.4944	2.7014	1.5459	0.0000
24-Aug-13	811.69	15.0	153.4944	0.9452	0.3094	-0.6372
30-Aug-13	811.69	30.0	153.4944	1.8648	0.5746	0.0000
31-Aug-13	811.69	25.0	153.4944	2.0265	0.8398	0.0000
1-Sep-13	811.69	45.0	153.4944	0.9590	0.3094	-0.6372
6-Sep-13	811.69	3.0	153.4944	2.2630	0.9716	0.0000
18-Sep-13	811.69	90.0	153.4944	3.1728	1.7227	0.0000
19-Sep-13	811.69	40.0	153.4944	4.7361	3.0466	0.0000
20-Sep-13	811.69	65.0	153.4944	4.1895	2.3412	0.0000
21-Sep-13	811.69	40.0	153.4944	3.4523	1.5900	0.0000

22-Sep-13	811.69	0.5	153.4944	2.9548	1.1479	0.0000
23-Sep-13	811.69	0.0	153.4944	2.5180	0.7954	0.0000
24-Sep-13	811.69	0.0	153.4944	2.6898	0.7951	0.0000
25-Sep-13	811.69	0.0	153.4944	2.4137	0.7064	0.0000
26-Sep-13	811.69	0.0	153.4944	1.5473	0.2210	0.0000
27-Sep-13	811.69	0.0	153.4944	2.0485	0.8830	0.0000
28-Sep-13	811.69	0.0	153.4944	1.5300	0.5304	0.0000
29-Sep-13	811.69	0.0	153.4944	1.7659	0.6188	0.0000
30-Sep-13	811.69	20.0	153.4944	1.8457	0.6670	0.0000
1-Oct-13	811.69	20.0	153.4944	1.7564	0.5304	0.0000
2-Oct-13	811.69	0.0	153.4944	1.4121	0.4420	-0.3115
		Total Spill 2013		ASSAG	88.32	
30-Aug-14	811.69	35.0	153.4944	2.2676	1.7667	0.0000
31-Aug-14	811.69	46.4	153.4944	2.1113	1.6785	0.0000
1-Sep-14	811.69	86.0	153.4944	2.3838	1.6252	0.0000
2-Sep-14	811.69	34.6	153.4944	2.5467	1.6345	0.0000
3-Sep-14	811.69	34.2	153.4944	1.8461	0.7514	0.0000
4-Sep-14	811.69	20.0	153.4944	1.3911	0.7072	0.0000
5-Sep-14	811.69	6.2	153.4944	1.3547	0.6630	0.0000
6-Sep-14	811.69	18.0	153.4944	2.4015	1.6354	0.0000
7-Sep-14	811.69	18.0	153.4944	3.2619	3.0852	0.0000
8-Sep-14	811.69	22.0	153.4944	2.5320	2.1216	0.0000
9-Sep-14	811.69	30.0	153.4944	2.4249	1.1931	0.0000

10-Sep-14	811.69	6.0	153.4944	2.2994	1.3697	0.0000
11-Sep-14	811.69	10.0	153.4944	2.1008	1.1927	0.0000
12-Sep-14	811.69	0.0	153.4944	1.6182	0.3978	0.0000
13-Sep-14	811.69	0.0	153.4944	1.5916	0.8398	0.0000
14-Sep-14	811.69	0.0	153.4944	1.3386	0.6630	0.0000
15-Sep-14	811.69	30.4	153.4944	0.6876	0.2652	0.0000
16-Sep-14	811.69	4.0	153.4944	0.6514	0.2210	0.0000
		Total Spill 2014			21.81	
16-Sep-17	811.65	9.0	153.1829	2.6590	1.0596	0.3115
17-Sep-17	811.69	107.0	153.4944	8.0598	6.7713	0.0000
18-Sep-17	811.69	133.0	153.4944	3.4362	2.1856	0.0000
19-Sep-17	811.69	38.0	153.4944	2.2031	0.9272	0.0000
20-Sep-17	811.69	20.0	153.4944	1.3828	0.0883	0.0000
21-Sep-17	811.69	8.0	153.4944	1.7770	0.4850	0.0000
		Total Spill 2017	S.S.S.		11.52	
28-Jul-18	811.65	40.0	153.1829	1.7327	0.7735	0.3115
29-Jul-18	811.69	40.0	153.4944	1.8630	1.2150	0.0000
30-Jul-18	811.69	10.0	153.4944	1.6425	0.9945	0.0000
31-Jul-18	811.69	35.0	153.4944	3.5380	2.8900	0.0000
1-Aug-18	811.69	70.0	153.4944	2.1753	1.4136	0.0000
2-Aug-18	811.69	5.0	153.4944	1.7670	0.4860	0.0000
3-Aug-18	811.69	13.0	153.4944	2.0175	0.7290	0.0000
4-Aug-18	811.69	7.0	153.4944	1.5665	1.0601	0.0000

5-Aug-18	811.69	20.0	153.4944	0.7621	0.0442	0.0000
6-Aug-18	811.69	15.0	153.4944	1.9738	0.8398	0.0000
7-Aug-18	811.69	45.0	153.4944	3.1194	2.2449	0.0000
8-Aug-18	811.69	100.0	153.4944	13.9960	12.7000	0.0000
9-Aug-18	811.69	210.0	153.4944	11.4925	10.2134	0.0000
10-Aug-18	811.69	50.0	153.4944	6.5734	5.2774	0.0000
11-Aug-18	811.69	30.0	153.4944	6.2754	4.9800	0.0000
12-Aug-18	811.69	20.0	153.4944	10.0708	8.7752	0.0000
13-Aug-18	811.69	60.0	153.4944	11.8071	10.6563	0.0000
14-Aug-18	811.69	70.0	153.4944	30.7607	29.4647	0.0000
15-Aug-18	811.69	160.0	153.4944	36.3981	35.1027	0.0000
16-Aug-18	811.69	300.0	153.4944	44.4898	43.4600	0.0000
17-Aug-18	811.69	160.0	153.4944	40.2615	38.9800	0.0000
18-Aug-18	811.69	50.0	153.4944	34.6533	33.3895	0.0000
19-Aug-18	811.69	35.0	153.4944	20.5184	22.1035	-2.8603
20-Aug-18	811.38	30.0	150.6341	4.7057	8.6712	-5.2534
21-Aug-18	810.77	20.0	145.3807	6.9680	5.6729	0.0000
22-Aug-18	810.77	20.0	145.3807	2.3338	1.0387	0.0000
23-Aug-18	810.77	5.0	145.3807	4.7711	2.0600	1.4160
24-Aug-18	810.92	4.0	146.7967	3.6299	1.0608	1.2744
25-Aug-18	811.07	0.0	148.0711	3.3751	1.0608	1.0195
26-Aug-18	811.19	0.0	149.0906	1.8453	1.0608	-0.5097
27-Aug-18	811.13	1.5	148.5809	2.6052	0.5304	0.7788

23-Sep-18	811.53	0.0	151.9934	2.9380	3.0022	-1.3593
24-Sep-18	811.38	15.0	150.6341	3.2321	7.1899	-5.2534
25-Sep-18	810.77	10.0	145.3807	0.6664	1.1693	-1.7983
26-Sep-18	810.59	0.0	143.5824	1.2225	1.0608	-1.1328
27-Sep-18	810.46	15.0	142.4496	0.6232	0.1768	-0.8496
Total Spill 2018					301.55	

Table 2.2 – Spill details

2.2.2 Design Flood and Features Related to Safety

Hydrology i) Catchment:

The Sholayar River is one of the main tributaries of Chalakudy river, the other important tributaries being Parambikulam, Thekkadi, Karapara and Anakayam. The river rises in the escarpment of the Western Ghats and Anamalai hills, at an elevation of about 1219.20 m and flows in a westerly direction for about 45.05 km through a plateau and then turns towards north and flows rapidly to join Parambikulam river at Orukembankutty at an elevation of about 457.20 m. About one mile downstream of the confluence of the Sholayar river and Parambikulam river, the Karapara joins the main river and from this point onwards the river is known as Chalakudy river which after a meandering course of 144.81 km joins the northern arm of Periyar and finally empties into the back waters of the Arabian Sea.

The river from its source flows within Madras state for about 24.14 km and for another 8.85 km along the boundary limits of Kerala and Madras and from the point where it enters Kerala State it flows for another 27.35 km and joins Parambikulam Ar.

The catchment area of the river above the Poringalkuthu Falls, where the Poringalkuthu Left Bank H. E. Scheme is projected is about 957.89 Sq.km and the catchment above the Sholayar dam site is 186.48 Sq.km of which 121.73 Sq. km lie within

Madras State. An agreement has been reached between the two states regarding the utilisation of waters of Sholayar. The text of the agreement is as follows.

"Madras will build a storage dam on Sholayar. Madras will start filling the Sholayar reservoir in the Kerala State from 1st July and keep it filled to near full reservoir level as soon as possible. Normally their reservoir should be filled by end of August. Also the Kerala reservoir should be at the Full Reservoir Level on 31st January every year. Kerala will withdraw total quantity of 348.29 Mm³ every year measured every year at the Sholayar Power House site."

The entire catchment receives the benefit of both the South west and North east monsoons. The South west monsoon sets in June and lasts till August. The heaviest rainfall is during the month of June and July. The North east monsoon strikes in October and continues till November. A heavy annual rainfall, a humid atmosphere and fairly uniform temperature throughout the year are the characteristic features of the basin.

ii) Yields:

The annual rainfall varies from 330.20 cm at Poringalkuthu to 457.20 cm and over in the Anamalai and Nelliampathi hills which are at higher elevations. The lowest rainfall recorded in the catchment during the period from 1948 to 1953 is 271.78 cm in 1952 while it is 403.86 cm in 1948 (both the figures are the average of the several rain gauge stations in the entire catchment area). The weighted average rainfall of the catchment for the above 6 years works out to 321.31 cm. The average annual flow in the catchment of Poringalkuthu reservoir and Sholayar reservoir are assessed to be 1982.18 Mm³ and 396.44 Mm³ respectively from the rainfall data collected in the years 1948 to 1953.

River discharge measurements using weir floats or current meters were not available for the overall design of the project. Only after the start of the project discharge measurements were arranged to be taken.

iii) Storage:

The storage required has been analysed by working tables and it is found from a number of working tables prepared that the best results are obtained with a reservoir

capacity of 150.08 Mm³ and a firm power draft of 10.76 cumecs. Due allowance for evaporation loss and for any possible silting has been made in the working tables. For this storage, the FRL of the reservoir has been fixed at +811.68 m. It has been found that any further raising of the reservoir would not make power generation any more cheaper. It is, therefore proposed to limit the FRL to +811.68 m

iv) Water Spread:

Water spread area of the reservoir formed by Sholayar main dam, Flanking dam and Saddle dam at F.R.L is 8.705 sq. km.

Design Flood

Clear flood marks to compute the maximum flood were not available at the time of preparation of the project report and so the maximum discharge was calculated from Ryve's formula (Q = C $M^{2/3}$) where Q = Flood discharge in cusecs, C = a constant and M = area of catchment. The value of C for 47 miles (75.62 km) of catchment lying within Madras state where the rainfall is comparatively high is assessed as 3000 while for the balance 25 miles (40.23 km) catchment up to the Sholayar Dam is assessed as 2500. Therefore the maximum flood discharge is calculated as 3000 x $47^{2/3}$ + 2500 x $25^{2/3}$ = 60400 Cusecs (1710.32 cumecs).

From 1959 onwards correct flood discharges have been recorded from the gauging weir at dam site. During normal year the flood discharge is in the order of 368.12 to 481.38 cumees. It is calculated that high peak flood of nearly 1698.99 cumees can occur in an exceptionally rainy year.

Hydrology Review

As per BIS 11223-1985, the dam is classified under large dams, i.e., Reservoir Capacity above 60Mm3 and Hydraulic head above 30m. For large dams, spillways are to be designed for PMF.

Revised Design Flood

The design storm value of one day PMP of 48.51 cm has been adopted for both sub catchments. The unit hydrograph of Sholayar Dam (Tamil Nadu) as derived by CWC has been used in the study while the unit hydrograph of free catchment has been derived using

FER sub zone 5 (a) and 5 (b). The time distribution of 24 hours rainfall has been taken from the PMP Atlas. The loss rate of 0.1 cm/hr., base flow of 0.15 m3/sec/sq. km have been adopted from CWC FER sub zone 5 (a) and 5 (b) in the study. The duration of design storm has been adopted as 24 hours. The peak rate of inflow flood hydrograph at Upper Sholayar Dam (Tamil Nadu) has been assessed as 1731 m³/sec. reservoir and channel routed 20 km to the brink of Sholayar reservoir (Kerala) using Modified Puls, Muskingum method. Reservoir routing was carried in the portion of inflow flood hydrograph in which inflow was higher than spillway capacity at FRL. The reservoir was initially assumed to be at FRL. The total design flood at Sholayar Dam (Kerala)has been assessed by adding channel routed flood from Upper Sholayar Dam (Tamil Nadu) and flood hydrograph from the free catchment at Sholayar Dam (Kerala). The peak of PMF at Sholayar Dam (Kerala) works out as **1890 m³/sec**.

2.3A Sholayar Flanking Dam

A flanking dam was required to be constructed to close the low saddle existing at the left flank of main dam to complete the periphery of the reservoir at its maximum water level. The flanking dam is located about 0.20 km south of the main dam. The spillway of Sholayar reservoir is located in this dam. The dam is built with rubble masonry and concrete, the total volume of which comes to 0.044 Mm³. The dam is having gravity type non overflow sections at both ends and overflow section in the spillway portion from Ch. 68.88 m to 137.16 m. The axis of the dam has a bend at Ch. 144.78 m and the alignment makes an angle of 40° to the axis of the dam up to this chainage. The bed level of the river near the flanking dam site is +768.10 m. The dam is 259.08M long at road level which is at an elevation of +812.90 m. The maximum height above the deepest foundation level is 27.74 m, width at the top is 4.27 m.



Fig 2.3 Sholayar Flanking Dam u/s view

2.3.1 A Components

Spillway

Spillway is of solid gravity type with the crest and downstream face curved to conform to flow of water. Clear water way provided is 5 spans of 10.97 m and including 2.44 m thick piers, the overall length of spillway is 68.27 m. The crest level is +805.28 M. The high level and low level buckets are separated by a dividing wall. The lip angle is kept at 35^o for reducing extra height of side walls.

5 Nos. of Radial Gates of size 10.97 m x 6.40 m are installed for controlling the flood discharge. The embedded parts of the radial gates consist of sill beams of I-section of welded construction and anchorages, sliding plates of sides of angle section with anchorages. The cross beams at piers are made of box type steel. These cross beams support the trunnions of the gates. The main body of the gates consists of skin plates and stiffeners of plate angles. There are two main girders of welded I-section and two arms made out of I-section having hubs trunnions and bearing body.

The Spillway is designed for a maximum designed flood discharge of 1710.32 cumecs. Dissipation of energy of the water falling from the ogee crested spillway is effected by providing up turned buckets with inverted levels at +786.99 m and +791.56 m. The radius and lip angle provided for the upper bucket are 10.67 m and 45^o respectively, while the corresponding values for the lower bucket are 12.19 m and 45^o. The dam is made accessible over the spill way portion by the spillway bridge and the motors of the radial gates, the winches and the control panels are located on the hoist bridge at R.L. +815.95 m



Fig 2.4 Spillway of Sholayar Flanking Dam

Foundation gallery

In Sholayar Flanking Dam, a foundation gallery is provided. The finished section of the gallery is 2.13 m x 1.52 m, a semi circle 1.52 m dia. surmounted on a rectangle 1.37 m x 1.52 m. It has a total horizontal length of 135.94 m. This include two transverse galleries for access. Beyond the galleries, on either end, drain culverts of 0.76 m x 0.76 m size for a total length of 64 m have been provided. The gallery has a longitudinal slope of 1 in 50.

There are 20 Porous Holes and 20 Foundation Drain Holes. Monthly observations of these holes are being carried out. Total seepage is measured using V-notch fitted in the gallery.

For measuring uplift pressure, Pressure gauges are used for measuring the uplift in the gallery and the readings are taken monthly.

Salient Features of Spillway

Sl. No	Feature	Description			
i	Type of gate	Spillway Crest Radial gate			
ii.	Size of Gate	10.97 m x 6.40 m			
 111.	No. of Gates	5 Nos.			
iv.	Clear width of opening	10970 mm			
v.	Operation Arrangement	a) Electrical Operation b) Manual Operation			
vi.	Gate position Indicator	Mechanical Dial Indicator at the control panel			
vii	Crest level of spillway	+ 805.28 m			
viii	Sill level of Gate	+ 805.28 m			
ix	Top of Gate	+ 811.68 m			
X	FRL	+ 811.68 m			
xi	MWL	+ 811.68 m			

Table 2.3 Salient Features of Spill way

Hoisting mechanism:

The hoisting mechanism consists of a central drive and two lateral drives coupled by pipe shafts. The central drive has spur type reduction gear, self - arresting worm gear, a double shoe brake with electro hydraulic control device. The hoist can be either electrically or manually operated. For hand operation there is a hand crank and a safety device for switching off electrical system when the hand crank is introduced. The lateral drums have got spur type reduction gears and a rope drum. There is a positive indicator in the control panel for indicating the extent of opening of the gates. It has got two concentric needles, one for indicating the opening in metres and the other for fraction of metre ie. centimetres. There are electrically controlled limit switches for the top and bottom positions of the gate. The hoisting and lowering speed when electrically operated is 0.31 m per minute. The electrical equipment consists of 3 phase synchronise motor with short circuit rotor 2.5 kilo watts, 1400 rpm magnetic hydraulic brake unit, spindle limit switches for top and bottom position of the gates. The motors of the radial gates, the winches and the control panels are located on the hoist bridge.



Fig 2.5 Radial gate Hoists

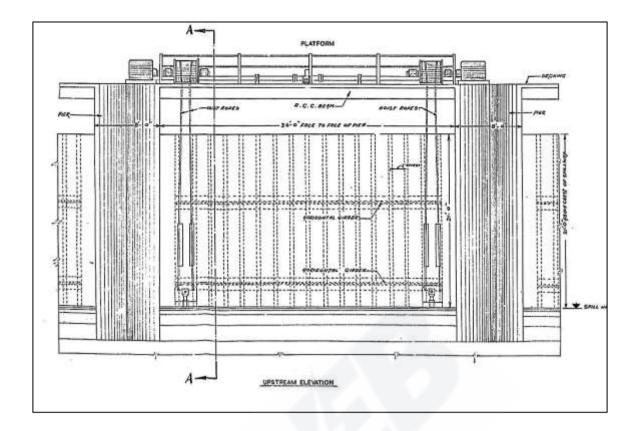


Fig 2.6 Hoisting arrangement



Fig 2.7 Control panel of Hoisting mechanism

River outlet Works

There are no river outlet works for Sholayar Flanking Dam

2.3 B Sholayar Main Dam

The Main Dam built across the Sholayar river is designed as a non-overflow straight gravity dam and is constructed in rubble masonry in cement mortar varying in proportion 1:3 to 1:5. The total quantity of concrete and masonry contained in this dam is 0.30 Mm³. The deepest foundation level is +746.46 m and the RL at the top of Dam is +812.60 m. The height of the dam above the deepest foundation is 66.14 m, length of the dam being 430.53 m at the road level. The width at the deepest foundation is 55.78 m. The width of dam at top is 7.32 m.

There are no overflow arrangements in this dam. For the purpose of getting water down the river for emergencies, an outlet pipe of 1.83 m dia. is placed at elevation +768.10 m is at 12.19 m above the bed level. The outlet has usual emergency gate and hoists and a jet disperser.



Fig 2.8 SHOLAYAR MAIN DAM

2.3.1 B Components

i) Drainage and inspection gallery

A drainage gallery has been constructed along the full length of the dam so as to accommodate the curtain grout holes on the upstream edge of the gallery and drain holes on the downstream edge. The drainage gallery is 1.52 m x 2.74 m in size in the river bed portion between chainages 126.03 m and 220.45 m and 1.52 m x 2.13 m in size for the remaining portion. The height of the gallery is 1.4 times the width. But in between chainages 126.03 m and 251.78 m due to the steepest slope of the gallery and owing to the reason that grouting may become necessary at a later date due to the poor nature of the foundation when compared to other blocks, the height of the gallery was increased by 0.61 m. The sides are vertical with a semi-circular top for the roof.

A transverse gallery of size 1.52 m x2.74 m in size is located in Block No.3 with an ADIT house at entrance at the downstream side of the dam. This is only entrance and exit provide for the drainage gallery.

The drainage holes in the gallery are located at the downstream face of the gallery and the water coming up through these are led out to the longitudinal drain on the upstream face of the gallery through cross drains. There are 90 Porous Drain Holes and 76 Foundation Drain Holes. Monthly observations of these holes are being carried out. Total seepage is measured using V-notches fitted in the gallery.



Fig 2.9 Drainage and inspection gallery

ii) Outlet Arrangements

The outlets from the reservoir are through the spillway arrangements at flanking dam and through the outlet arrangement at main dam. The spillway will discharge water only up to +805.28 m level. When water level goes below that level and in case water is to be fed into Poringalkuthu reservoir without passing through the turbines, this is achieved through the disperser arrangement provided at main dam.

The disperser outlet arrangement is provided at chainage 161.54 m on the main dam in Block 4 at elevation +768.10 m and consists of the following.

- 1. A trash rack for preventing trashes getting inside the disperser arrangement.
- 2. Emergency gate and hoist for closing and opening of outlet arrangement.
- 3. A 1.83 m dia., pipe from the upstream face to the downstream face with a taper piece at the downstream end.
- 4. Disperser (hollow jet).

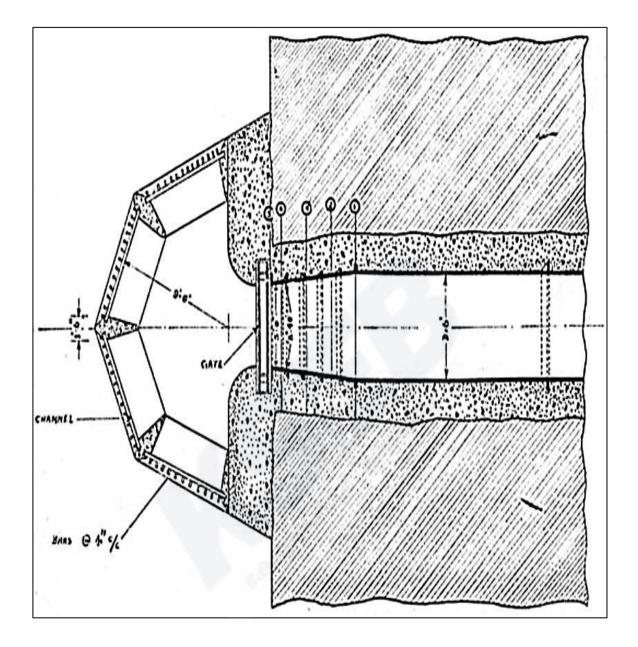


Fig 2.10 Plan of outlet arrangement

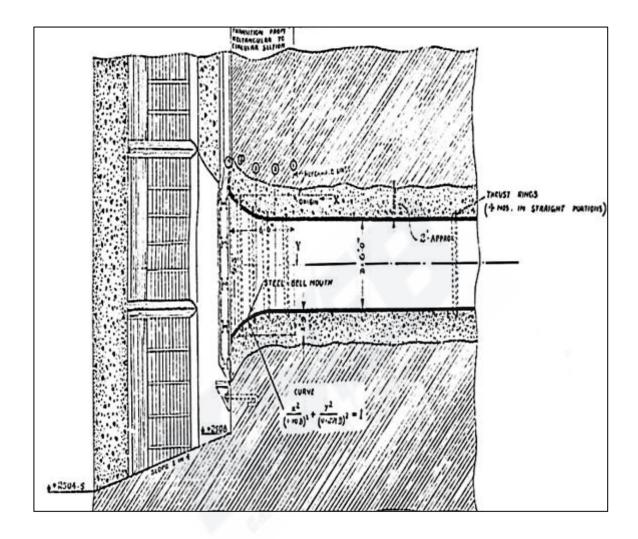


Fig 2.11 Section of outlet arrangement

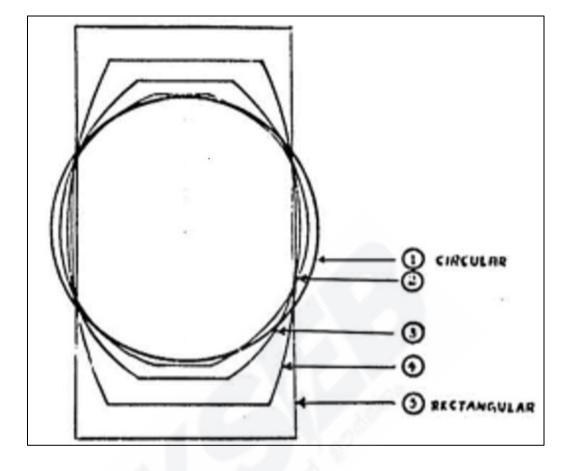


Fig 2.12 Transition Sections

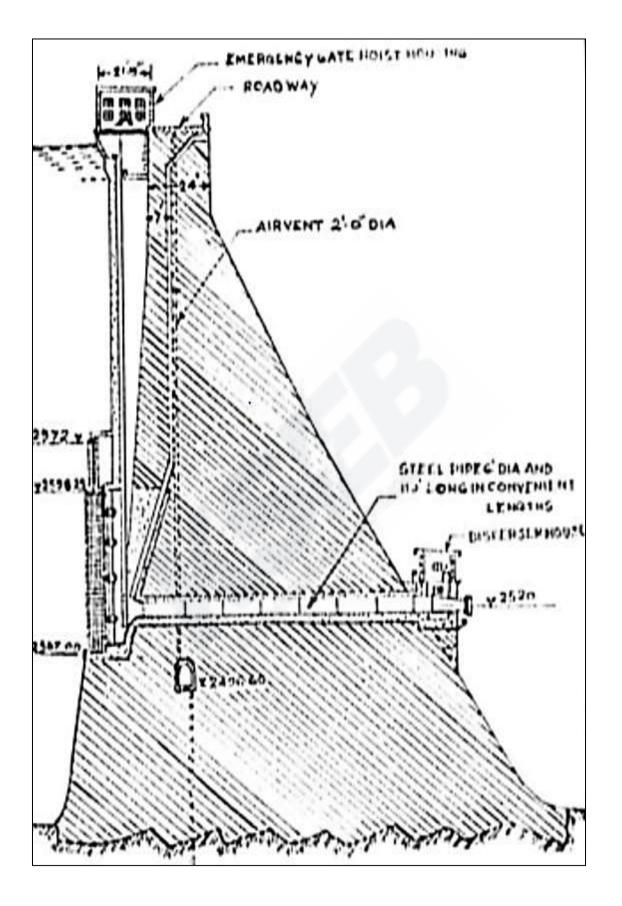
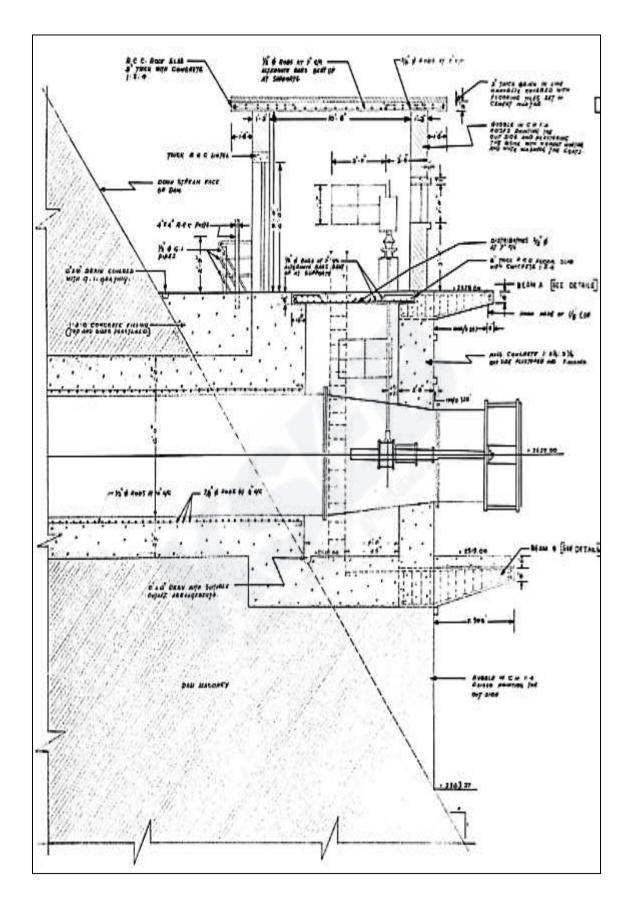
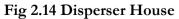


Fig 2.13 Section of Main Dam through center line of Outlet Pipe





iii) Emergency gate and hoist

Emergency gate and hoist are supplied by M/S WaagnerBire, Austria, through their Indian Agents M/S Marshall Sons & Co. Ltd., Madras. The gate is designed for a clear opening of 2.90 m x 1.52 m. Wheels with antifriction bearing, self aligning axles etc. enable good alignment of the gate.

There is a mechanical hoist erected at elevation +812.67 m with capacity of 20 tons for operating the gate. The hoist consists of electric motor, solenoid brake and a rope drum. There is a fan brake of sturdy construction, independently acting under different brake torques. There is a suspension beam for the hoist gate, which moves along with the gate during its downward movement, so that the gate is supported by 2 ropes, till it reaches the bottom near the opening. Beyond this, the suspension beam is held in position and from there the gate is suspended by 4 ropes, so that the effort on the motor is reduced at the higher reaches and maximum effort is put when the gate during its upward and downward movements. There is a position indicator attached to the hoist frame work, which indicates the position of the gate and also there are two limit switches for limiting its upward and downward movements. The hoisting speed is 1.52 m per minute and maximum lowering speed is 2.44 m per minute.



Fig 2.15 Rope drum of Emergency gate



Fig 2.16 Control panel of Emergency Gate

iv) Pipe arrangements

There is a bell mouth at the entrance varying from rectangular section of 3.23 m to circular section of 1.83 m dia., and from there a steel pipe 1.83 m dia. and 1.27 cm thick and 36.27 m long is provided for carrying water. At the end there is a taper piece varying in section from 1.83 m dia. to 1.22 m dia. 2.44 m long connecting to the hollow jet. The bell mouth, pipes and taper piece are supplied by M/S Structural Engineering Works, Bombay.

v) Disperser

The disperser or discharge regulator for the outlet arrangement is supplied by M/S Neyrpic, France through their Indian Agents M/S Batliboi & Co. Pvt. Ltd., Bombay. The hollow jet valve consists of the following.

- Body: This is made of welded steel plate and consists of an upstream flanged cylinder connected to the valve cone at the downstream end by means of radial ribs. The annular space is between the cylinder and the valve cone is the valve outlet. Both the radial ribs and the cylinder are carefully machined to provide guides and sliding surfaces for the bronze shoes on the valve sleeve.
- 2. Valve Sleeve: This consists of a cylinder made of stainless steel plate. The sleeve is well stiffened by means of annular and radial ribs which are also used for attaching the two operating rods. The sleeve is fitted with carefully machined bronze shoes which guide it and upon which it slides on the valve body.
- 3. **Seals**: Downstream end of the valve outlet is fitted with an elastic seal. Another seal attached to the body prevents upstream leakage between the body and the valve sleeve inner surface of stainless steel.
- 4. **Operating gear**: The valve is operated by 2 worm gears located in horizontal plane passing through the axis of the valve and consists of a chrome steel work, steel bevel gear and bronze driving nut. The operating gear gives only longitudinal motion to the worm. The worm gears are driven by the motor unit

via shafts and bevel gearing. There is a mechanical position indicator with limit contacts, which indicates the opening of the gate. The valve can also be manually operated in the case of power failure. There is a safety device for cutting off the electric supply when the crank handle is introduced. The maximum discharge through the discharge regulator under full head is 29.34 cumecs. The center of outlet pipe and disperser is fixed at + 768.10 m level.



Fig 2.17 Operating gear of Disperser valve



Fig 2.18 Motor of opening arrangement of disperser valve

5. **Trash rack arrangements** : 5 rows of trash racks of size 3.12 m x 1.88 m are provided in front of the outlet arrangement for preventing trashes entering the outlet pipe. The trash bars are fixed at 10 cm centers. Necessary grooves are formed in the concrete at the time of construction of piers and a channel section made out of 1.27 cm thick plate is fixed inside the groove to form as a guide for the movement of the trash rack at the time of its removal and insertion. Dowel bars are provided in between trash racks for getting correct alignment and seating.

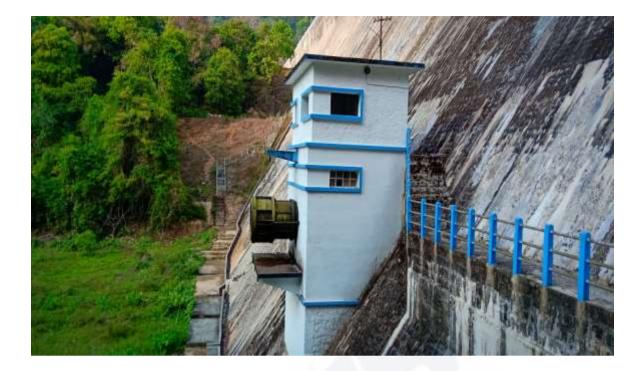


Fig 2.19 Disperser valve

2.3 C Sholayar Saddle Dam

The Composite Saddle Dam at Ambalapara is the third dam constructed under Sholayar Hydro Electric Project to form the Sholayar Reservoir. This dam is located in the Ambalapara Saddle between Sholayar and Anakkayam valleys. The dam is of composite type with an upstream masonry wall and earth embankment backing. The masonry portion is founded on sound rock while the earth embankment is formed over the excavation for the masonry portion and on the original surface with 0.30 to 0.60 m of the surface earth alone removed. The total quantity of concrete and masonry contained in this dam is 0.008 Mm³. The quantity of earth fill for the embankment is 0.01 Mm³.

The deepest foundation level for masonry portion is +795.22 m and the RL at the top of Dam is +813.82 m. Height of the dam above the deepest foundation of masonry portion is 18.59 m, length of the dam at top is 109.12 m. Top width of masonry portion is 1.83 m and earth embankment is 6.10 m.



Fig 2.20 SHOLAYAR SADDLE DAM

2.4 Operation of the Reservoir

Sholayar reservoir was being operated as per "Guidelines for Operation of Reservoirs" (IS;7323;1994), for storage reservoirs for conservation purposes like power generation. The reservoir water is released through spillway gates as per rule levels fixed. Discharge through single spillway for different reservoir levels with different gate openings is tabulated in **Table 2.4**. Discharge (Rating) curve showing discharge through one radial gate at various reservoir levels is given in **Fig. 2.21**. Total discharge in full opened position is given in **Fig. 2.22**.

Reservoir level (Feet)		2663	2662	2661	2660	2659	2658	2657	2656	2655
Reservoir level (Metre)		811.68	811.38	811.07	810.77	810.46	810.16	809.85	809.55	809.24
One Gate opening in Feet	One Gate opening in Metre	Discharge in M³/s								
0.10	0.0305	2.47	2.41	2.35	2.29	2.23	2.16	2.09	2.02	1.94
0.20	0.0610	4.94	4.82	4.70	4.57	4.44	4.31	4.17	4.03	3.88
0.30	0.0914	7.40	7.22	7.04	6.85	6.66	6.46	6.25	6.03	5.81
0.40	0.1219	9.86	9.62	9.37	9.12	8.86	8.59	8.32	8.03	7.73
0.50	0.1524	12.31	12.01	11.70	11.39	11.06	10.72	10.38	10.02	9.65
0.60	0.1829	14.75	14.39	14.02	13.64	13.25	12.85	12.43	12.00	11.56
0.70	0.2134	17.19	16.77	16.34	15.89	15.44	14.97	14.48	13.98	13.46
0.80	0.2438	19.62	19.14	18.65	18.14	17.62	17.08	16.52	15.95	15.35
0.90	0.2743	22.05	21.51	20.95	20.38	19.79	19.18	18.55	17.91	17.23
1.00	0.3048	24.47	23.87	23.25	22.61	21.95	21.28	20.58	19.86	19.11
1.10	0.3353	26.89	26.22	25.54	24.84	24.11	23.37	22.60	21.80	20.98
1.20	0.3658	29.29	28.57	27.82	27.05	26.27	25.45	24.61	23.74	22.84
1.30	0.3962	31.70	30.91	30.10	29.27	28.41	27.53	26.62	25.67	24.69
1.40	0.4267	34.09	33.24	32.37	31.47	30.55	29.60	28.61	27.60	26.54
1.50	0.4572	36.48	35.57	34.63	33.67	32.68	31.66	30.60	29.51	28.38
1.60	0.4877	38.87	37.89	36.89	35.86	34.81	33.71	32.59	31.42	30.21
1.70	0.5182	41.24	40.21	39.14	38.05	36.92	35.76	34.56	33.32	32.03
1.80	0.5486	43.62	42.52	41.39	40.23	39.04	37.80	36.53	35.21	33.84
1.90	0.5791	45.98	44.82	43.63	42.40	41.14	39.84	38.49	37.10	35.65
2.00	0.6096	48.34	47.12	45.86	44.57	43.24	41.87	40.45	38.97	37.45
2.10	0.6401	50.70	49.41	48.09	46.73	45.33	43.89	42.39	40.84	39.24
2.20	0.6706	53.04	51.69	50.31	48.88	47.41	45.90	44.33	42.71	41.02
2.30	0.7010	55.38	53.97	52.52	51.03	49.49	47.90	46.26	44.56	42.79
2.40	0.7315	57.72	56.24	54.73	53.17	51.56	49.90	48.19	46.41	44.56
2.50	0.7620	60.05	58.51	56.93	55.30	53.62	51.89	50.10	48.25	46.32
2.60	0.7925	62.37	60.77	59.12	57.43	55.68	53.88	52.01	50.08	48.07

2.70	0.8230	64.69	63.02	61.31	59.55	57.73	55.86	53.92	51.90	49.81
2.80	0.8534	67.00	65.27	63.49	61.66	59.77	57.82	55.81	53.72	51.54
2.90	0.8839	69.30	67.51	65.66	63.76	61.81	59.79	57.70	55.53	53.27
3.00	0.9144	71.60	69.74	67.83	65.86	63.84	61.74	59.58	57.33	54.99
3.10	0.9449	73.89	71.97	69.99	67.96	65.86	63.69	61.45	59.12	56.69
3.20	0.9754	76.18	74.19	72.14	70.04	67.87	65.63	63.31	60.90	58.40
3.30	1.0058	78.46	76.40	74.29	72.12	69.88	67.56	65.17	62.68	60.09
3.40	1.0363	80.73	78.61	76.43	74.19	71.88	69.49	67.02	64.45	61.77
3.50	1.0668	83.00	80.81	78.57	76.26	73.87	71.41	68.86	66.21	63.45
3.60	1.0973	85.26	83.01	80.70	78.31	75.86	73.32	70.69	67.96	65.12
3.70	1.1278	87.51	85.20	82.82	80.37	77.84	75.22	72.52	69.71	66.78
3.80	1.1582	89.76	87.38	84.93	82.41	79.81	77.12	74.34	71.44	68.43
3.90	1.1887	92.00	89.55	87.04	84.45	81.77	79.01	76.15	73.17	70.07
4.00	1.2192	94.24	91.72	89.14	86.48	83.73	80.89	77.95	74.89	71.70
4.10	1.2497	96.46	93.88	91.23	88.50	85.68	82.77	79.74	76.60	73.33
4.20	1.2802	98.69	96.04	93.32	90.52	87.62	84.63	81.53	78.31	74.94
4.30	1.3106	100.90	98.19	95.40	92.53	89.56	86.49	83.31	80.00	76.55
4.40	1.3411	103.11	100.33	97.47	94.53	91.49	88.34	85.08	81.69	78.15
4.50	1.3716	105.32	102.47	99.54	96.52	93.41	90.19	86.85	83.37	79.74
4.60	1.4021	107.51	104.60	101.60	98.51	95.32	92.02	88.60	85.04	81.33
4.70	1.4326	109.70	106.72	103.65	100.49	97.23	93.85	90.35	86.70	82.90
4.80	1.4630	111.89	108.84	105.70	102.47	99.13	95.67	92.09	88.36	84.46
4.90	1.4935	114.06	110.95	107.74	104.43	101.02	97.49	93.82	90.01	86.02
5.00	1.5240	116.23	113.05	109.77	106.39	102.90	99.29	95.54	91.64	87.57
5.10	1.5545	118.40	115.15	111.80	108.35	104.78	101.09	97.26	93.27	89.11
5.20	1.5850	120.56	117.24	113.82	110.29	106.65	102.88	98.97	94.89	90.64
5.30	1.6154	122.71	119.32	115.83	112.23	108.51	104.66	100.67	96.51	92.16
5.40	1.6459	124.85	121.39	117.83	114.16	110.37	106.44	102.36	98.11	93.67
5.50	1.6764	126.99	123.46	119.83	116.09	112.21	108.21	104.04	99.71	95.17
5.60	1.7069	129.12	125.53	121.82	118.00	114.05	109.97	105.72	101.29	96.66
5.70	1.7374	131.25	127.58	123.81	119.91	115.89	111.72	107.38	102.87	98.15
5.80	1.7678	133.37	129.63	125.78	121.81	117.71	113.46	109.04	104.44	99.63
5.90	1.7983	135.48	131.67	127.75	123.71	119.53	115.20	110.69	106.00	101.09
6.00	1.8288	137.58	133.71	129.72	125.60	121.34	116.92	112.34	107.55	102.55
6.10	1.8593	139.68	135.74	131.67	127.48	123.14	118.64	113.97	109.10	104.00
6.20	1.8898	141.77	137.76	133.62	129.35	124.93	120.36	115.60	110.63	105.44
6.30	1.9202	143.86	139.77	135.56	131.22	126.72	122.06	117.21	112.16	106.87

6.40	1.9507	145.94	141.78	137.50	133.07	128.50	123.76	118.82	113.68	108.29
6.50	1.9812	148.01	143.78	139.42	134.92	130.27	125.44	120.42	115.19	109.70
6.60	2.0117	150.08	145.77	141.34	136.77	132.03	127.12	122.02	116.69	111.10
6.70	2.0422	152.13	147.76	143.26	138.60	133.79	128.80	123.60	118.18	112.49
6.80	2.0726	154.19	149.74	145.16	140.43	135.54	130.46	125.18	119.66	113.87
6.90	2.1031	156.23	151.71	147.06	142.25	137.28	132.11	126.74	121.13	115.25
7.00	2.1336	158.27	153.68	148.95	144.07	139.01	133.76	128.30	122.59	116.61
7.10	2.1641	160.30	155.64	150.84	145.87	140.73	135.40	129.85	124.05	117.97
7.20	2.1946	162.33	157.59	152.71	147.67	142.45	137.03	131.39	125.50	119.31
7.30	2.2250	164.34	159.54	154.58	149.46	144.16	138.66	132.92	126.93	120.64
7.40	2.2555	166.35	161.48	156.44	151.24	145.86	140.27	134.45	128.36	121.97
7.50	2.2860	168.36	163.41	158.30	153.02	147.55	141.88	135.96	129.78	123.29
7.60	2.3165	170.36	165.33	160.15	154.79	149.24	143.47	137.47	131.19	124.59
7.70	2.3470	172.35	167.25	161.99	156.55	150.92	145.06	138.97	132.59	125.89
7.80	2.3774	174.33	169.16	163.82	158.30	152.58	146.65	140.45	133.98	127.17
7.90	2.4079	176.31	171.06	165.64	160.05	154.25	148.22	141.93	135.36	128.45
8.00	2.4384	178.28	172.95	167.46	161.78	155.90	149.78	143.41	136.73	129.72
8.10	2.4689	180.24	174.84	169.27	163.51	157.54	151.34	144.87	138.09	130.97
8.20	2.4994	182.20	176.72	171.08	165.23	159.18	152.89	146.32	139.45	132.22
8.30	2.5298	184.14	178.60	172.87	166.95	160.81	154.42	147.77	140.79	133.45
8.40	2.5603	186.09	180.46	174.66	168.65	162.43	155.96	149.20	142.13	134.68
8.50	2.5908	188.02	182.32	176.44	170.35	164.04	157.48	150.63	143.45	135.89
8.60	2.6213	189.95	184.17	178.21	172.04	165.65	158.99	152.04	144.77	137.10
8.70	2.6518	191.87	186.02	179.98	173.73	167.24	160.50	153.45	146.07	138.30
8.80	2.6822	193.78	187.86	181.74	175.40	168.83	161.99	154.85	147.37	139.48
8.90	2.7127	195.69	189.69	183.49	177.07	170.41	163.48	156.24	148.65	140.65
9.00	2.7432	197.59	191.51	185.23	178.73	171.98	164.96	157.62	149.93	141.82
9.10	2.7737	199.48	193.32	186.96	180.38	173.54	166.43	158.99	151.19	142.97
9.20	2.8042	201.37	195.13	188.69	182.02	175.10	167.89	160.36	152.45	144.11
9.30	2.8346	203.25	196.93	190.41	183.66	176.64	169.34	161.71	153.70	145.25
9.40	2.8651	205.12	198.73	192.12	185.28	178.18	170.78	163.05	154.93	146.37
9.50	2.8956	206.98	200.51	193.83	186.90	179.71	172.22	164.39	156.16	147.48
9.60	2.9261	208.84	202.29	195.52	188.51	181.23	173.64	165.71	157.38	148.58
9.70	2.9566	210.69	204.06	197.21	190.11	182.74	175.06	167.03	158.59	149.67
9.80	2.9870	212.53	205.82	198.89	191.71	184.25	176.47	168.33	159.78	150.75
9.90	3.0175	214.36	207.58	200.56	193.29	185.74	177.87	169.63	160.97	151.81
10.00	3.0480	216.19	209.33	202.23	194.87	187.23	179.26	170.91	162.14	152.87

i i	1	1	1	1	1	1	1	1	1	
10.10	3.0785	218.01	211.07	203.89	196.44	188.71	180.64	172.19	163.31	153.92
10.20	3.1090	219.83	212.80	205.54	198.00	190.18	182.01	173.46	164.47	154.95
10.30	3.1394	221.63	214.53	207.18	199.56	191.64	183.37	174.72	165.61	155.97
10.40	3.1699	223.43	216.24	208.81	201.10	193.09	184.72	175.97	166.75	156.99
10.50	3.2004	225.22	217.95	210.44	202.64	194.53	186.07	177.20	167.87	157.99
10.60	3.2309	227.01	219.66	212.05	204.17	195.97	187.40	178.43	168.98	158.98
10.70	3.2614	228.78	221.35	213.66	205.69	197.39	188.73	179.65	170.09	159.95
10.80	3.2918	230.55	223.04	215.26	207.20	198.81	190.05	180.86	171.18	160.92
10.90	3.3223	232.31	224.72	216.86	208.70	200.22	191.35	182.06	172.26	161.88
11.00	3.3528	234.07	226.39	218.44	210.20	201.61	192.65	183.25	173.33	162.82
11.10	3.3833	235.81	228.05	220.02	211.68	203.00	193.94	184.42	174.39	163.75
11.20	3.4138	237.55	229.71	221.59	213.16	204.39	195.22	185.59	175.44	164.67
11.30	3.4442	239.28	231.36	223.15	214.63	205.76	196.48	186.75	176.48	165.58
11.40	3.4747	241.01	233.00	224.70	216.09	207.12	197.74	187.90	177.51	166.47
11.50	3.5052	242.72	234.63	226.25	217.54	208.48	198.99	189.04	178.53	167.36
11.60	3.5357	244.43	236.26	227.78	218.99	209.82	200.23	190.17	179.53	168.23
11.70	3.5662	246.13	237.87	229.31	220.42	211.16	201.46	191.28	180.53	169.09
11.80	3.5966	247.83	239.48	230.83	221.85	212.48	202.69	192.39	181.51	169.94
11.90	3.6271	249.51	241.08	232.34	223.26	213.80	203.90	193.49	182.48	170.77
12.00	3.6576	251.19	242.68	233.85	224.67	215.11	205.10	194.57	183.45	171.60
12.10	3.6881	252.86	244.26	235.34	226.07	216.41	206.29	195.65	184.39	172.41
12.20	3.7186	254.53	245.84	236.83	227.46	217.70	207.47	196.71	185.33	173.21
12.30	3.7490	256.18	247.41	238.31	228.85	218.98	208.64	197.77	186.26	173.99
12.40	3.7795	257.83	248.97	239.78	230.22	220.25	209.80	198.81	187.18	174.77
12.50	3.8100	259.47	250.52	241.24	231.58	221.51	210.96	199.85	188.08	175.53
12.60	3.8405	261.10	252.06	242.69	232.94	222.76	212.10	200.87	188.97	176.27
12.70	3.8710	262.73	253.60	244.13	234.29	224.01	213.23	201.88	189.85	177.01
12.80	3.9014	264.34	255.13	245.57	235.62	225.24	214.35	202.88	190.72	177.73
12.90	3.9319	265.95	256.65	247.00	236.95	226.46	215.46	203.87	191.58	178.44
13.00	3.9624	267.55	258.16	248.42	238.27	227.68	216.56	204.85	192.42	179.13

Table 2.4 Spill way Discharge

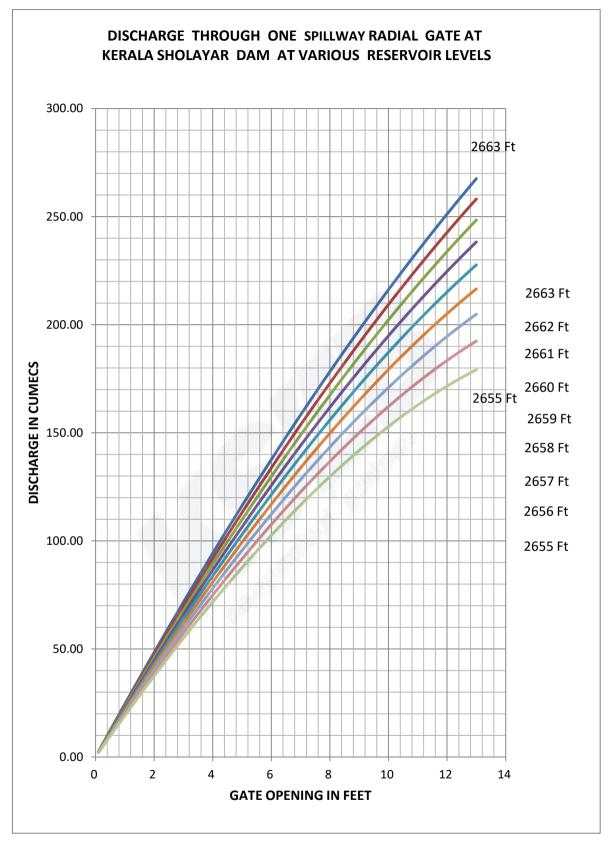


Fig 2.21 Discharge curve

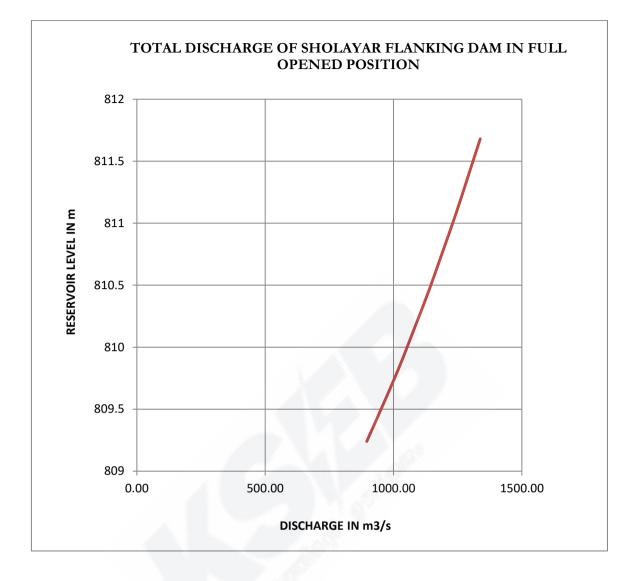


Fig. 2.22 Total Discharge in full opened position

2.4.1 Rule Curve

As per the Kerala flood study report of August 2018, CWC has recommended for reviewing the rule curves of all the reservoirs in Kerala. The rule curves need to be formulated for both conservation as well as operations during the flood, in case of storage reservoirs also, particularly for the reservoirs having the live storage capacity of more than 200 Mcm in order to create some dynamic flood cushion for moderating the floods of lower return periods particularly in the early period of monsoon. Sholayar reservoir comes under the system of reservoirs of Parambikulam- Aliyar Project. Maintenance of water levels in this reservoir, withdrawal/release from reservoir etc. are to be done in compliance with the Parambikulam- Aliyar Project Agreement executed between Kerala and Tamil Nadu.

- As per PAP agreement, Tamil Nadu Government shall let down water from Tamil Nadu Sholayar reservoir to Kerala Sholayar reservoir to ensure the following:
- Tamil Nadu shall commence filling Sholayar Reservoir from 1st July and fill it up to 2658feet (810.16m) as soon as possible.
- Sholayar reservoir shall be kept at the full reservoir level of 2663 feet (811.68m)on September 1st.
- From September 2nd to January 31st, the level in Sholayar Reservoir shall be maintained at about 5 feet below FRL ie. 2658 feet (810.16m).
- On February 1st, the Sholayar reservoir shall be kept at full reservoir level of 2663feet (811.68m).

During February to June 30th, no releases need to be made from Tamil Nadu reservoir provided that 12.3 TMC of water had been delivered at the Sholayar Power House by 1st February.

As such rule levels are already there for Sholayar reservoir. Review of the levels can be done only with the consensus of Joint Water Regulatory Board. Even then, based on quality prediction of rainfall, the water level in the reservoir can be lowered to 0.50 m below the FRL during August & October without conflicting with PAP agreement considering the trend of inflow in the preceding month.

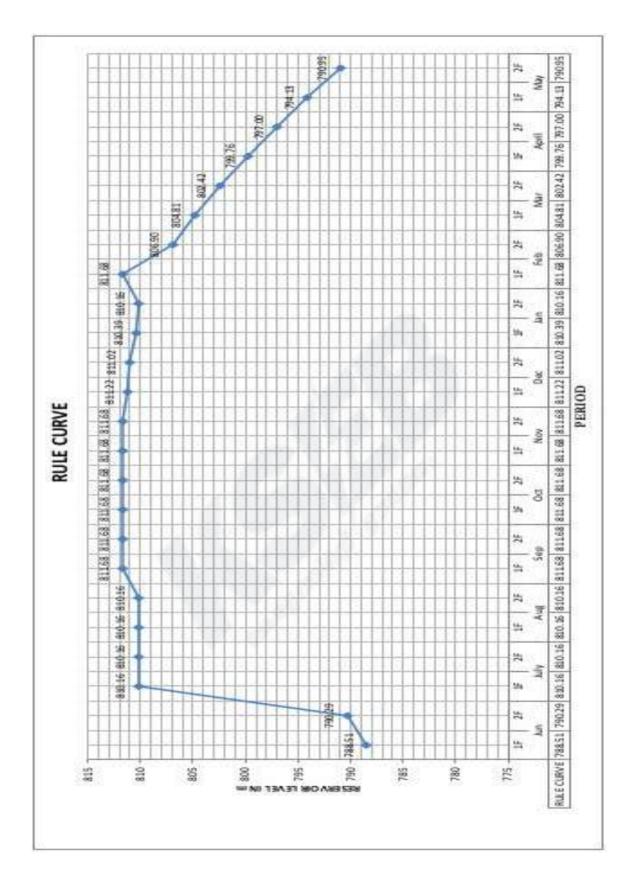
Accordingly Rule curve for Sholayar Flanking Dam is given in **Fig. 2.23**. The reservoir water exceeding the rule curve level will be spilled or adjusted with power generation. This can be used till further revision.

Month/ Year	Ju	June		July		August		September		ober	November	
	1F	2F	1F	2F	1F	2F	1F	2F	1F	2F	1F	2F
1999-00												
2000-01	783.15	787.51	792.94	799.13	802.69	807.85	811.43	811.28	811.54	811.20	810.39	809.2
2001-02	787.17	790.89	797.55	803.86	805.55	807.63	810.27	809.38	809.76	809.68	809.42	808.8
2002-03	783.42	786.99	792.37	793.14	794.45	799.59	803.88	804.30	804.44	807.66	809.83	809.8
2003-04	779.91	782.56	789.29	794.43	799.26	803.41	807.07	807.48	807.96	810.20	809.76	810.1
2004-05	789.51	797.25	800.25	804.56	809.77	810.79	810.99	809.51	809.06	810.07	810.62	810.4
2005-06	789.44	787.90	792.02	799.67	811.40	811.45	811.66	811.61	810.94	810.70	809.73	810.4
2006-07	793.70	792.62	797.09	803.98	808.33	811.29	811.07	811.16	810.85	811.60	811.64	810.7
2007-08	788.27	788.51	798.03	808.11	811.17	811.67	811.68	811.68	811.14	811.13	811.50	811.3
2008-09	787.81	788.81	792.00	794.26	801.80	807.02	810.76	811.37	811.32	811.44	811.26	811.1
2009-10	786.68	784.40	789.16	802.14	807.26	810.18	811.61	811.47	811.59	811.31	810.85	811.4
2010-11	791.02	790.78	793.53	799.19	805.92	810.39	811.40	810.88	810.82	810.81	811.06	810.7
2011-12	792.57	794.41	796.83	803.64	809.22	811.47	811.67	811.63	810.81	810.89	811.43	811.2
2012-13	790.30	788.14	789.75	795.08	801.16	806.94	810.87	811.42	810.70	810.94	811.03	810.8
2013-14	784.18	790.09	800.46	807.14	811.65	811.64	811.48	811.64	811.55	811.38	811.10	810.6
2014-15	782.10	785.04	787.39	795.09	805.16	810.44	811.68	811.57	811.30	811.27	811.34	810.5
2015-16	787.52	787.53	892.01	796.92	802.92	807.60	810.95	811.45	811.27	810.31	809.66	807.8
2016-17	787.43	787.64	789.96	797.75	802.54	806.27	808.62	808.06	808.59	809.55	809.74	809.0
2017-18	782.30	783.89	790.24	795.94	800.82	805.35	810.20	811.66	811.16	810.59	811.10	810.6
2018-19	792.02	797.28	800.79	810.10	811.68	811.28	811.47	811.07	809.52	810.32	810.19	809.9
Average	787.29	789.07	799.03	800.22	805.41	808.54	810.46	810.45	810.23	810.58	810.61	810.2

SHOLAYAR DAM - W L FROM 2000 TO 2018													
Year/Month	December		January		February		March		April		May		
	1F	2F	1F	2F	1F	2F	1F	2F	1F	2F	1F	2F	
1999-00			804.53	800.89	797.09	793.85	791.51	789.23	787.94	786.77	786.85	784.17	
2000-01	808.49	808.60	809.11	808.79	806.93	805.46	803.67	801.50	798.17	796.02	792.77	789.31	
2001-02	808.18	806.58	804.63	802.25	800.04	797.37	794.15	790.55	788.48	787.29	784.36	783.04	
2002-03	809.44	808.04	806.28	805.89	805.77	803.96	801.96	799.70	796.66	793.01	789.32	784.60	
2003-04	810.23	810.40	808.87	807.13	805.00	802.50	799.76	797.25	794.59	791.15	789.04	789.00	
2004-05	810.71	810.85	810.54	810.64	810.81	808.81	806.83	804.73	802.41	800.11	797.44	793.53	
2005-06	809.85	809.20	809.30	810.78	811.29	809.67	807.57	804.99	801.55	798.80	796.30	793.56	
2006-07	810.24	809.95	808.24	809.30	809.64	807.99	806.00	803.13	799.36	795.87	792.69	790.36	
2007-08	811.18	810.45	809.63	808.66	807.22	805.28	803.39	801.65	799.85	796.93	793.37	789.26	
2008-09	810.88	811.30	811.35	811.40	810.88	808.72	806.06	803.88	801.45	798.62	795.23	791.03	
2009-10	811.31	811.08	811.09	811.35	810.97	809.31	807.53	805.35	803.01	800.54	797.99	794.74	
2010-11	810.61	810.62	811.06	811.55	810.53	808.29	806.30	804.12	801.68	799.22	796.31	792.54	
2011-12	810.52	810.37	811.29	811.58	810.80	809.06	807.18	804.98	802.64	800.23	797.69	794.33	
2012-13	810.80	810.77	809.11	807.44	805.15	803.67	801.96	799.92	797.57	794.48	791.84	788.73	
2013-14	811.05	811.42	811.25	810.92	810.60	808.79	806.95	804.27	800.37	796.05	792.45	787.57	
2014-15	810.31	810.70	810.94	811.24	810.94	809.20	807.04	804.24	801.01	797.80	793.84	791.76	
2015-16	806.47	807.29	808.93	809.05	807.38	806.18	804.25	801.96	800.05	797.39	793.99	790.30	
2016-17	809.35	808.87	807.90	806.68	805.06	803.45	801.65	799.16	795.83	792.61	789.15	784.65	
2017-18	810.75	810.50	810.25	809.28	807.85	806.28	804.43	802.23	799.61	797.02	794.65	792.38	
2018-19	809.68	809.13											
Average	810.00	809.80	809.17	808.67	807.58	805.68	803.59	801.20	798.54	795.78	792.91	789.73	
Rule curve	811.22	811.02	810.39	810.16	811.68	806.90	804.81	802.42	799.76	797.00	794.13	790.95	

Table 2.5 Reservoir Water Levels

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2.4.2 Safety Aspects

The spillway gates are operated step by step after assessing the reservoir water level and inflow and the sequencing is defined in **2.4.3**.

2.4.3 Flood Release Procedure

The flood water is released through spillway gates based on the operation manual of gates and flood routing studies attached in the hydrology review **Annexure 2**. There are five spillway gates. The sequence of operation of spillway gates is Gate no. 3, 2, 4, 1, 5 ie. The spillway gate No. 3 is operated first. Thereafter depending on requirement gates on either side of the central gate nos. 2 & 4 are opened. Spillway gates 1 & 5 are operated last. Further opening of gates, if required is done in the same way, keeping the difference in the openings of any two adjacent gates not more than 0.2m. Closing of gates is to be done in reverse order; the gate opened last being closed first.

2.4.4 Reservoir Capacities



Fig 2.24 Sholayar Reservoir

The Gross storage of the reservoir is 153.48 Million Cubic Meters and the effective Storage is 150.08 Million Cubic Meters at FRL of +811.68m.

2.4.5Inflow forecasting

The project was conceived based on the rainfall details from 1948 to 1953. The lowest rainfall recorded in the catchment during the period from 1948 to 1953 is 271.78 cm in 1952 while it is 403.86 cm in 1948 (both the figures are the average of the several rain gauge stations in the entire catchment area). The weighted average rainfall of the catchment for the above 6 years works out 321.31 cm. The average annual flow in the catchment of Poringalkuthu reservoir and Sholayar reservoir are assessed to be 1982.18 Mm³ and 396.44 Mm³ respectively from the rainfall data collected in the years 1948 to 1953.

Sholayar entire catchment receives the benefit of both the South west and North east monsoons. The South west monsoon sets in June and lasts till August. The heaviest rainfall is during the month of June and July. The North east monsoon strikes in October and continues till November. A heavy annual rainfall, a humid atmosphere and fairly uniform temperature throughout the year are the characteristics features of the basin.

Based on the recent flood, a flood forecasting method is proposed to be implemented. During years of heavy monsoon, incessant rains in the catchment area and releases from Upper Sholayar dam of Tamil Nadu are likely to cause flood in Sholayar River. These floods may lead to problems like people getting displaced from their homes, huge damage to crops and other assets. The floods can have disastrous impact on the environment also. Adequate measures are required to be taken up in advance to control and regulate the flow of water in the river. The regulation of flood water released from the upstream dam in Tamil Nadu is important and has an impact in Sholayar flood. The following measures are essential for effective management of floods in Sholayar River during the monsoons.

- 1) Nomination of liaising officers for respective reservoirs.
- 2) Sharing of Sub-basin wise directory of concerned officers responsible for flood management.
- 3) Exchange of data regarding rainfall, releases from dams, reservoir water levels.

4) Reservoir operation schedules.

5) Exchange of data regarding rainfall, releases from dams, reservoir water levels.

During monsoon, daily water releases from the Dams/Barrages at 8 hrs. & 16 hrs.in normal situation and hourly data exchange during heavy floods is necessary.

Flood warning system in catchment area: The regulation of flood from the dam on upstream of Sholayar reservoir is very crucial to quantify the inflow of floods into Sholayar dam. The dam operators of Upper Sholayar Project managed by Tamil Nadu and Kerala Sholayar dam shall work with good coordination and pass on information so as to assist in the flood forecasting. Accordingly, sequence of opening or closing of gates will be taken up in accordance with the approved gate operation schedule.

Flood communication system: The widely time tested Communication to reach every corner of the flood affected zones have been radio and television and private media for the people to move to safer places by themselves in an emergency. Communication is very important in such occasions. These days due to revolution in the telecommunication system, networks of mobile phones are widely available. Advantage of this facility will be taken. Mobile numbers of all officers and staff working in Dam operations wing, District Disaster Management, Revenue authorities will be listed and made available to all the personnel who have been assigned duty of disaster management. A satellite phone is provided at the dam site.

2.4.6 Methodology of Flood Regulation at Sholayar

2.4.6.1 Inflow Computation

Inflow into reservoirs is normally estimated by the reservoir gauging method (also called the rise and fall method or inflow-outflow method). All the outflows are added together and to it the rate of rise in storage (Positive if the level rises, and negative if it falls) is added. Expressed as an equation, this will be.

Inflow (cumecs) = Total outflow (cumecs) + Rate of increase in storage (cumecs)

The rate of increase or decrease in storage can be determined from the observed rate of increase or decrease in reservoir level and the elevation capacity tables. For easy computation a table can be developed showing the rate of change of storage in the Sholayar reservoir for a rate of rise in reservoir level of 1 cm/hour. This table can be put to use for

easy interpolation. Once the inflow is known the outflow and gate opening required to maintain the water level can be computed.

2.4.7 Summary of Flood Regulation Procedure

The flood regulation procedures at Sholayar can be summarized in the following 5 steps:

- 1. Observe the reservoir level at 1 hour intervals.
- Determine the total outflow occurring at all outlets (including river outlet, spillway, power Intake)
- 3. Estimate the inflow
- 4. Determine the gate opening as the case maybe.
- 5. Open the required number of gates to the extent required to maintain constant reservoir level (i.e., release is equal to the inflow) as per the upper rule curve.

2.4.8 Emergency Operation

The purpose of Emergency Action Plan is to identify emergency situations that could threaten Sholayar Dams and to plan for an expedited, effective response to prevent failure of the dam and warn downstream residents of impending danger. This plan defines the notification procedures to be followed in the event of a potentially hazardous situation. The procedures are intended to protect lives and prevent property damage from and excessive release of water from the dam spillways or an uncontrolled outflow of water from the breached portion of dam.

Dam owner's responsibilities before and during an Emergency event, Dam Engineers Preparedness& Responsibilities, Responsibilities for Notification, Responsibilities for Evacuation, Responsibilities for Termination and Follow-Up, Communication Networks, Emergency Detection, Evaluation and Classification, Preparedness, Remedial Actions, Emergency Operations Centre, Inundation Areas, Local Evacuation Plan, Implementation, Vicinity Map Inundation cum Evacuation Maps etc. will be provided in the detailed EAP document of Sholayar Dams. The Emergency operation will be carried out following the Emergency Action Pan (EAP). The Emergency conditions are outlined in Chapter 4 under clause **4.2.1** on Immediate Maintenance. The EAP together with this Manual will be present at site at all times.

2.5 Power Generation

In the Power House, 3 vertical Francis type turbines each of 31000 HP coupled to 3 generators of 18000 KW capacity each are installed. The manufacturers are M/S Titovi Zavodi Litostroj, Yugoslavia.

2.6 Tunnel Intake arrangements

The intake arrangement comprises of:

- 1. Trash rack structure
- 2. A steel lined tunnel from the intake point to the intake shaft.
- 3. The intake shaft
- 4. Emergency gate and hoisting arrangements.

Trash rack structure is extended out into the river and from that point of the bell mouth, 28.49 m long, 3.20m dia. steel pipe starts. Beyond that there is 27.43 m long tunnel and there is a transition from tunnel section to rectangular section near the gate for a length of 6.10 m. At this point intake shaft is constructed vertically for a height of 46.63 m with the hoist chamber and control arrangements above it. The hoist is erected on the top slab of the hoist chamber constructed above intake shaft at El. +819.91 m. The hoist chamber is used for the maintenance of intake gate. There is a position indicator fixed to the hoist, which indicates the position of the gate from the sill.



Fig 2.25 Intake gate hoisting structure



Fig 2.26 Wire rope and motor in intake gate hoist chamber

As per the original design, only 10.20 m length of intake pipe was provided and orders were placed with M/S Thungabhadra Steel Product Ltd. But later on it was decided to push the intake structure 18.29 m more into the river and for that additional length, orders were placed with M/S Giovanola Binny Ltd. All the pipes were erected in position. For the steel lining, there is a bell mouth at the front which has got a size of 5.03 m x 14.81 m changing into 3.20 m circular shape at the rear. From that point the dia. of pipe is 3.20 m throughout. The steel lining has got a total length of 28.48 m.

2.6.1 Intake gate and Hoisting Arrangements

The order for the supply of intake gate and hoist was placed with M/S VOEST, Austria. The size of the gate is 2.87 m (span) x 3.96 m (height) suitable for the opening provided in the structure of size 3.66 m x 2.44 m. The sill level of the gate is +773.32 m and the hoist was erected at +825.70 m. The hoist is operated by an electric motor of 12KW at 725 rpm. Provision is given for operating the hoist manually in case of power failure and safety provisions are made to cut off power supply when manual operation is done. The lifting speed of the gate is 1.52 m per minute when electrically operated and 10.16 cm per minute when manually operated. The lowering speed under emergency which is by gravity is 2.44 m per minute. In emergency lowering the free drop of the gate is being stopped by a brake electrically operated. The gate by means of a gear operates a D.C. generator which produces available current according to speed of the fall and this actuates the brake. The details of intake gate hoist are provided in **Annexure 6**.



Fig 2.27 Intake Gate

2.7 Power Tunnel and Surge Shaft

The total length of high pressure tunnel from its mouth at the reservoir end to surge shaft is 710.49 m. It is having a horse shoe shape and a sectional area of 7.90sqm. The tunnel is designed to carry a maximum flow of 23.36 cumecs. The designed velocity of flow through the tunnel is 2.96 m/second. The level of the inlet of the tunnel at the intake is +773.32 m and there is general slope of 1 in 120 towards the surge shaft. Control of flow into the tunnel is achieved by an intake gate erected inside the intake shaft. The tunnel is lined with cement concrete to an average thickness of 20.32 cm for the sides and the bottom.

The surge shaft driven through rock has a diameter of 9.14 m at the bottom up to elevation +809.55 m and 15.24 m up to elevation +816.86 m. There is horizontal surge gallery of length 60.96 m and section area 13 sq.m. From the surge shaft the low pressure pipe line starts with its diameter 3.20 m and runs for a length of 71.32 m.

2.8 Penstocks

The water from the reservoir is diverted to the Power House at Anakayam Valley through the tunnel and through 3 penstocks, each leading the water to one of the turbines in the Power House. The penstocks are running in exposed conditions and are made of steel plates of thickness varying from 10 mm to 32 mm. The internal diameter of the penstocks is 1438 mm at the top, ie. at the point where the penstocks branches from the Trifurcation piece and is 1312 mm dia. at the Power House end. Penstocks supply water under pressure at an average head of 320.04 m and form the link between the fore bay and the Power Station.



Fig 2.28 Penstock and valve house

2.9 Initial Filling of Reservoir

The initial filling of the reservoir was carried out during 1965.

2.10 Power House

The Power House is located at the right bank of the Anakayam stream, a tributary of Chalakudy river. It is located at the bottom of the ridge through which the penstock is taken. 3 Vertical Francis type turbine each of 31000 HP coupled to 3 generators of 18000 KW

capacity each are installed in the Power House. Size of the Power House is $50.29 \text{ m} \times 14.02 \text{ m} \times 16.76 \text{ m}$. The maximum head on turbines is 339.24 m.

The first unit was commissioned in May 1966, though the benefits of the scheme have actually been started to be enjoyed with partial storages of the reservoir during the previous two seasons for generation of additional power in the Poringalkuthu Power Station.



Fig 2.29 Power House



Fig 2.30 Power House Turbine



Fig 2.31 Power House Control panels

2.10.1 E.O.T. Crane at Power House

The E.O.T. crane has got a main hook of 75 ton capacity and an auxiliary hook of 10 ton capacity. The crane booms are fixed on end carriages which are resting over the crane rails fixed on wooden sleepers. The centre to centre distance between the crane rails is 12.882m. The booms are of box type with rails on the top on which the carriage can move.

A hand operated 20 ton crane is provided outside the Power House for handling of the spherical valves. This crane is running on the runway rails fixed over the gantry girders erected on the top of the columns on the hill side of the Power House. The crane is capable of moving for a length of 31.70 m to command the three spherical valves.

2.11 Record Keeping

The records regarding dam and appurtenant structures including detailed drawings and construction details are kept at the field office. Essential documents as per the dam safety guidelines are kept at dam site office.

Following records of reservoir operations are being maintained:

- 1. Rainfall record on daily basis throughout the year.
- 2. Reservoir levels on daily basis during non-monsoon and hourly basis during monsoon.
- 3. Depth of outflow over the spillway on hourly basis during monsoon.
- 4. Estimated spillway outflows during monsoon on hourly basis.
- 5. Power releases.
- 6. Water audit register to be maintained for estimating the inflows on hourly basis during monsoon and daily basis during non-monsoon by accounting all the releases/outflows and the incremental change in storage in the reservoir.

Chapter 3 Project Inspection

An effective inspection program is essential to identify problems and to keep a dam in a good and healthy condition. Inspection details and suggestions are kept at field office and reports are sent to higher offices. The current practice of Inspection at Sholayar dam envisages that the Deputy Chief Engineer along with Executive Engineer of the project shall carryout pre-monsoon and post-monsoon inspections as per CWC guidelines in the format issued by CWC. The Chief Engineer will review the inspection report and submit to CWC. The Executive Engineer of the Project will also conduct quarterly inspections and will prepare health reports as per KDSA. The format to be followed as per CWC has been revised during January 2018 and new guidelines have been issued vide Doc No. CDSO_GUD_DS_07_v1.0, CWC 2018 for Safety Inspection of Dams. The health reports of the dam are to be prepared in the new format meant for incorporating the data in DHARMA. Detailed description on project inspections is available in the Guideline for Safety Inspection of dams. However an overview of the various types of inspections required to be carried out at Sholayar dam is given below. Note that for uploading Inspection Data into DHARMA, the Inspection Instructions & Forms given in the above mentioned Guideline for Safety Inspection of Dams must be used.

Guidance for carrying out other inspections is elaborated in the following paras:

3.1 Types of inspections

Four types of dam safety inspections could be carried out at Sholayar Dam. These include, but not limited, to the following:

- 1. Comprehensive evaluation inspections
- 2. Scheduled inspections (Pre & Post monsoon inspections & other scheduled inspections)
- 3. Special (Unscheduled) inspections
- 4. Informal inspections

The frequency of each type of inspection depends on the condition of the dam and State DSO regulations.

Typical inspection elements and the detail of the safety inspections are provided below. More detailed descriptions are given in the 'Guideline for Safety Inspection of Dams' (CWC 2018). A comprehensive health checklist for recording the status of each item being inspected and the overall condition of the equipment along with any consequential risks on the health of the dam is also to be maintained.

3.2 Comprehensive Evaluation Inspections

For comprehensive dam safety evaluation for each dam, an independent panel of experts known as Dam Safety Review Panel (DSRP) needs to be constituted for determining the condition of the dam and appurtenant works. The panel will undertake evaluation of the dam once in 10 years or on occurrence of any extreme hydrological or seismic event or any unusual condition of the dam or in the reservoir rim. The terms of reference of the comprehensive dam safety evaluation shall include but not be limited to:

- General assessment of hydrologic and hydraulic conditions, review of design flood, flood routing for revised design flood and mitigation measures.
- Review and analysis of available data of dam design including seismic safety, construction, operation maintenance and performance of dam structure and appurtenant works.
- Evaluation of procedures for operation, maintenance and inspection of dam and to suggest improvements / modifications.
- Evaluation of any possible hazardous threat to the dam structure such as dam abutment slope stability failure or slope failures along the reservoir periphery.

A comprehensive evaluation inspection of Sholayar dam consists of five major parts:

1. Review of project records (i.e. study of all design / construction records/drawings, history of the dam's performance, past inspection

notes/reports, notes on distress observed/ any rehabilitation measures undertaken earlier, instrumentation data and its interpretation including.

- 2. Inspection of the dam and its appurtenant works.
- 3. To review the results and reports of additional field investigations & laboratory testing as required.
- Review of design studies, review of design flood, checking of the adequacy of Spillway capacity, freeboard requirements, dam stability and any special study as required.
- 5. Preparation of a detailed report of the inspection.

3.2.1 Details to be provided to DSRP before inspection.

All relevant details / data / drawings for the dam project to be examined by the DSRP shall be provided at least 3 months in advance of the proposed visit. This will include:

- General information and scope of the project
- Emergency preparedness
- Details of key personnel
- Hydrology original and reviewed
- Reservoir operation and regulation plan
- Basic data and issues related to safety of dam
- Problems, if any, during construction
- Drawings of dam, spillway, gates and appurtenant structures
- Seismicity aspects & details
- Status of the instrumentation
- Construction history
- Geological report including special problems at site and their treatment
- Field Inspection- Observation and recommendation regarding remedial measures
- Dam incidents and reservoir filling details

3.3 Scheduled Inspections

Scheduled inspections shall consist of Pre-monsoon & Post-monsoon inspection and any other inspections carried out by the State Dam Safety Organization/any Expert panels constituted by the dam owner. These inspections are performed to gather information on the current condition of the dam and its appurtenant works. This information is then used to establish needed repairs and repair schedules, and to assess the safety and operational adequacy of the dam. Scheduled inspections are also performed to evaluate previous repairs.

The purpose of scheduled inspections is to keep the dam and its appurtenant structures in good operating condition and to maintain a safe structure. As such, these inspections and timely maintenance will minimize long-term costs and will extend the life of the dam. Scheduled inspections are performed more frequently than comprehensive evaluation inspections to detect at an early stage any developments that may be detrimental to the dam. These inspections involve assessing operational capability as well as structural stability and detection of any problems and to correct them before the conditions worsen. The field examinations should be made by the personnel assigned responsibility for monitoring the safety of the dam. If the dam or appurtenant works have instrumentation, the individual responsible for monitoring should analyze measurements, as and when the same are received and include in the evaluation report of that data. Dam Inspection Report or an inspection brief should be prepared following the field visit.

Scheduled inspections include the following components as a minimum:

- Review of past inspection reports, monitoring data, photographs, maintenance records, or other pertinent data as may be required
- Visual inspection of the dam and its appurtenant works
- Preparation of a report or inspection brief, with relevant documentation and photographs. The report should be filed in the dam owner's project files

3.3.1 Pre- and Post-Monsoon Checklist and Example of Reporting Proforma

Detailed checklists are to be followed to ensure evaluation of the health of the dam and to ensure that it continues to operate in satisfactory and safe condition. The proforma to be used for inspection should be the one enclosed in the Doc. No. CDSO.GUD.DS07 v 1.0, CWC 2018 on the Guide lines for Safety Inspection of Dams **(Annexure 3)**.

Performance of Dam instruments

Sl No	Name of Dam with location	Name of instrument	No of instruments	Performance	Status of data analysis	Remarks
1	2	3	4	5	6	7

Table 3.1 Proforma for performance of Dam instruments

The officers responsible for undertaking the Pre & Post Monsoon Inspections are as under:

i) Assistant Exe Engineer ii) Executive Engineer iii) Dy. Chief Engineer in-charge of dam in-charge of dam.

The information in regard to above inspection would be supplied to the Chief Engineer of the Project for further co-ordination with the concerned departments and district Authorities.

3.4 Special (Unscheduled) Inspections

Special inspections may need to be performed to resolve specific concerns or conditions at the site on an unscheduled basis. Special inspections are not regularly scheduled activities, but are usually made before or immediately after the dam or appurtenant works have been subjected to unusual events or conditions, such as an unusually high flood or a significant earthquake. These inspections are to be carried out by teams to be constituted by State DSO after an initial assessment based on informal inspection carried out by project personnel reveal dam safety related concerns like cracking in the dam, damages, erosion/ scour, undermining/ piping/ sink holes/ liquefaction or any such undesirable feature. A special inspection may also be performed during an emergency,

such as an impending dam breach, to evaluate specific areas or concerns. They are also made when the ongoing surveillance program identifies a condition or a trend that appears to warrant a special evaluation. Special inspections should focus on those dam components that are affected by the unusual event and should include at least three elements: 1) review of relevant files or data, 2) visual inspection, and 3) report preparation.

More detailed site investigations / studies may be required (such as drilling, surveys, or seepage flow estimates) if the special inspection reveals the need for the same. Photographic documentation is to be included as part of the inspection.

3.5 Informal Inspections

An informal inspection, is a continuing effort by on-site personnel (dam owners/operators and maintenance personnel) performed during their routine duties. Informal inspections are critical especially to keep an eye on the proper operation and maintenance of the dam. These inspections consist of frequent observations of the general appearance and functioning of the dam and appurtenant structures.

Operators, maintenance crews, or other staffs who are posted at Sholayar dam site are supposed to conduct informal inspections on routine basis. These people are the 'firstline of defense' in assuring safe dam conditions, and it is their responsibility to be familiar with all aspects of the dam. Their vigilance while walking across the dam for inspection / surveillance, checking the operating equipment, and noting changes in conditions may prevent serious mishaps or even dam failures.

Informal inspections are important and are performed at every available opportunity. These inspections may only cover one or two dam components as the case may be, or they may cover the entire dam and its appurtenant structures in one go. The informal inspections are not as detailed as comprehensive evaluation, scheduled, and special inspections and will only require that a formal report is submitted to the dam owner's project files if a condition is detected that might endanger the dam. Report is to be submitted detailing the condition discovered along with photographs, time, reservoir water level, other features etc.

Chapter 4

Project Maintenance

A good maintenance program is required to protect a dam against deterioration, prolong its life and reduce the chance of failure. Maintenance program for a dam should be developed primarily based on systematic and frequent inspections. Nearly all the components of a dam and its materials are susceptible to damage and deterioration if not well maintained. Moreover, the cost of a proper maintenance is small compared to the costs of major repairs, loss of life, property and litigation. If maintenance of a dam is neglected the consequences and costs could be enormous.

4.1 Maintenance Plan

A basic maintenance schedule for the various monitoring components based on manual of operating parts, frequent inspections, priority, and interval is attached as **Annexure 8**. This shows the tasks to be performed and how frequently that is to be inspected/observed and repaired.

4.2 Maintenance Priorities

Maintenance activities need to be prioritized. In order of priority they need to be clarified under the heads immediate maintenance & preventive maintenance.

4.2.1 Immediate Maintenance

The following conditions are critical and call for immediate attention & reservoir lowering, if warranted. These conditions may include, but are not limited to:

- The dam is about to be overtopped or being overtopped during high flood.
- The dam is about to be breached by erosion, slope failure etc.
- A dam showing signs of failure due to aging/cracking, sliding, overturning etc.
- The dam showing signs of piping or internal erosion along shear zones, faults etc. indicated by increasingly cloudy seepage or other symptoms.

- The spillway being blocked or with some inoperable gates.
- Evidence of excessive seepage as seen in the Gallery/on downstream face of the dam.

An EAP is to be activated when any of the above conditions are noted.

4.2.2 Preventive Maintenance

This can be further classified as Condition based Maintenance and Routine Maintenance.

4.2.2.1 Condition Based Maintenance

The following maintenance works are to be undertaken as soon as possible after the defective condition is noted. These include but are not limited to:

- Remove all vegetation and bushes by roots from the dam surface and restoring any eroded areas.
- Repair any concrete or metal components that have deteriorated.
- Cleaning of the choked drainage holes in the dam body/ foundations in concrete dams.
- Repair any damages on spillway glacis, piers, energy dissipaters, training/divide walls, downstream areas etc.
- Repairs on the upstream face of masonry dams, in case the pointing is damaged, due to which there is increased seepage.
- Controlling any heavy seepage in the foundation/ inspection galleries in Concrete/Masonry dams from drainage holes.
- Repairs of any cracks/cavities/joints in concrete/masonry dams/structures. However many of these works will require the services of experienced engineers/expert panels.

4.2.2.2 Routine Maintenance

Several tasks should be performed on a continuous basis. These include but are not limited to the following:

• Any routine repair to concrete or metal component.

- Observation of any springs or seepage areas, comparing the quantity and quality (clarity) with earlier observations.
- Monitoring of downstream development which could have an impact on the dam and its hazard category.
- Maintenance of Electrical & Hydro-Mechanical equipment and systems e. g. Servicing of spillway gates, hoisting arrangements and gates/hoist of Intake and outlet works & stand by generator.
- Maintaining proper lighting at dam top, galleries of the dam etc.
- Monitoring of seepage in foundation galleries of masonry/ concrete dams.
- Monitoring/ cleaning & removal of leached deposits in porous concrete / formed drains in dam body and foundation drainage holes of masonry/ concrete dams.
- Maintenance of all dam roads & access roads.
- Operation of electrical and mechanical equipment and systems including exercising gates & valves.
- To keep the gate slots clear of silt/debris.
- Maintenance/testing of monitoring equipment (instruments) and safety alarms.
- Testing of security equipment.
- Testing of communication equipment.
- Any other maintenance considered necessary.

4.3 Procedures for Routine Maintenance

4.3.1 Controlling Damage from Vehicular Traffic

Vehicles, except for maintenance, are restricted on the dam top and kept out by fencing or barricades. Any damages are repaired as soon as possible. Also vehicles are to be permitted only after security checking at check posts.

4.3.2 Controlling Vegetation

Removal of vegetation around the dam and its premises is done atleast2 times in a year.

4.3.3 Masonry / Concrete dams & spillways

The following important issues / aspects need to addressed while undertaking the periodic maintenance, but are not be limited to:

- Cracking in concrete (potential causes are alkali aggregate reaction, thermal stresses because of heat of hydration or temperature variations, foundation problems).
- Damages on spillway glacis, spillway piers, training/divide walls, energy dissipaters, downstream areas (probable causes are cavitation, abrasion, unsymmetrical flows, unfavorable down-stream conditions)
- Vegetation growth in spillways, spill channel, approach channel etc.
- Seepage in Galleries and on d/s face of the dam.
- Cleaning and removal of leached deposits from choked drainage holes in the dam body/foundations.
- Repair to upstream face of masonry dams in case the pointing is damaged, leading to increased seepage.
- Status of rectification works undertaken from time to time need to be assessed during periodic maintenance.
- To ensure proper access & lighting in galleries.
- To ensure that the dam is behaving as designed based on instrumentation programs.
- Periodic maintenance should be performed on all concrete surfaces to repair deteriorated areas. Repair of deteriorated concrete at the earliest following the standard specifications for repair of concrete surfaces and re-pointing of masonry joints etc. it is most easily repaired in its initial stages. Deterioration can accelerate and, if left unattended, can result in serious problems or dam failure.

For remedial measures of problems of special nature advice of experienced engineers/ Panel of Experts needs to be obtained.

4.3.4 Outlet Works

Proper care and maintenance of the equipment is the most important factor in increasing the reliability of operation and life of the equipment. Following are a few important points which need attention.

1. Lubrication of moving parts

All moving parts of the equipment are to be periodically lubricated.Lubrication schedule/chart is attached below for reference. During lubrication of all open gears, it must be noted that they are not in operation and there is no foreign materials/dirts on the surfaces to be lubricated.

2. Lubrication chart

SI. No.	Item	Recommended grade	Periodicity	Remarks
1.	Worm reducers of hoist	Servo Gear SM-90 of IOC or equivalent	Once in every year	Regular check or oil level is to be made and replenish with oil if necessary
2.	Hoists and other grease nipples	Servo Gear 2/3 of IOC or equivalent	Once in every three months	
4.	All pinion and gear wheels, guide shoes in the gate elements and lifting rollers	Servo Grease 'C' of IOC or equivalent	Once in every three months	
5.	Wire rope	Cardium compound	Once in every year, preferably before the onset of monsoon	

3. Preventive maintenance

While designing and manufacturing the equipment, sufficient care has been taken to use the correct materials and manufacturing process to avoid break down during operation.

a) <u>Electro Magnetic Brakes</u>

Worn out brake linings are to be replaced immediately.

b) Lifting Beam

For proper functioning of the lifting beam, lubrication of pins on the hook is a must. By properly greasing the pins, the hooks shall be free to oscillate without much effort. All wheels of the lifting beam are also to be lubricated. The operator is to ensure that both the hooks, are free to rotate for every successive operation.

c) <u>Guide Shoe</u>

Check the bolts and nuts periodically.

d)	Painting		
i)	Gates	1.27	All gates are to be properly repainted with two coats of Blend Epoxy Resin Paint once in every year. If any rusting is noticed, the rusted area is to be thoroughly cleaned and one coat of zinc rich primer is to be applied before applying epoxy paint.
ii)	Hoists		All parts of hoists are to be properly repainted with two coats of Aluminum paint once in every two years and this shall be done .preferably before the on set of monsoon.

Under water inspection of the trash rack provided in the outlet arrangement has to be done periodically and cleaning has to be carried out as and when required to remove the trashes trapped in the trash rack.

4.3.5 Power Tunnel & Trash Racks

The Power Tunnel is having an Intake gate with hoisting arrangement. The hoist is operated by an electric motor of 12KW at 725 rpm. Provision is given for operating the hoist manually in case of power failure. The lifting speed of the gate is 1.52 m per minute when electrically operated and 10.16 cm per minute when manually operated. The lowering speed under emergency which is by gravity is 2.44 m per minute. In emergency lowering the free drop of the gate is being stopped by a brake electrically operated. The gate by means of a gear operates a D.C. generator which produces available current according to speed of the fall. This actuates the brake.

Under water inspection of the trash rack provided in front of the Intake of Power tunnel has to be done periodically and cleaning has to be carried out as and when required to remove the trashes trapped in the trash rack.

4.3.6 Emergency Gate

An outlet arrangement is provided in Sholayar Main dam at elevation +768.10 m. The outlet has usual emergency gate and hoists and a jet disperser. The hoist is erected at elevation +812.67 m with capacity of 20 tons for operating the gate. The hoist consists of electric motor, solenoid brake and a rope drum. The Operation and Maintenance Manual of Emergency gate is given in **Annexure 6**.

4.3.7 Spillway Radial Gates & Hoisting Equipment

Sholayar Flanking Dam has 5 Radial gates for the spillway. Hoisting arrangements consist of a central drive and two lateral drives coupled by pipe shafts. The central drive has spur type reduction gear, self-arresting worm gear, a double shoe brake with electro hydraulic control device. The hoist can be either electrically or manually operated.

The safe and satisfactory operation of Sholayar Flanking Dam depends on proper operation of its Gates & Hoisting Equipment. Maintaining spillway gates in working condition is critical for dam safety and is to be assigned the highest priority. If routine inspection of the Hydro-Mechanical Equipment shows the need for maintenance, the work should be completed as soon as possible. The gates are to be operated through their full range twice annually (before monsoon & after monsoon keeping a gap of at least six months). Because operating gates under full reservoir pressure can result in large discharges, exercising of gates should preferably be carried out during dry conditions or lean times of the year.

The aspects to be inspected and maintained periodically for ensuring proper operation of gates in general are given below. The O&M manuals of the gates manufacturer's would however govern the overall maintenance of Gate and Hoists whenever there is any contradiction with the inspections given in the Manual.

- The gate slot and bottom platform/sill beam should be cleaned periodically. Scales formed over the embedded parts should be removed. Second-stage concrete should be checked for any development of cracks / leakages and repairs should be attended to immediately.
- ii) The gate leaf should be thoroughly cleaned and repainted as and when necessary according to the procedure or guidelines- indicated in IS: 14177 or as per the recommendations of the paint manufacturer. All drain holes provided in the gate assembly should be cleaned.
- iii) Rubber seals should be smoothened, if required, for proper alignment. All nuts and bolts fixing the seal to the gate should be tightened uniformly to

required torques. Seals, if found damaged or found leaking excessively should be adjusted, repaired or replaced as considered necessary.

- iv) The wheel shall be rotated to check their free movement. Gate roller bearings and guide roller bushes should be properly lubricated. Whenever necessary these should be opened for rectifications of defects, cleaning and lubrication and should thereafter be refitted. These may be replaced if repairs are not possible.
- v) Hoisting connection of the gate leaf should be lubricated where necessary and defects if any should be rectified.
- vi) All nuts, bolts, check nuts and cotter pins of the lifting devices should be checked periodically.
- vii) All components should be greased and lubricated. Recommended and approved oils and grease only should be used.
- viii) All welds shall be checked for cracks/ damages. Any weld that might have become defective should be chipped out and redone following the relevant codal provisions. Damaged nuts, bolts, rivets, screws etc. should be replaced without delay.
- ix) The guide-assemblies, wheel-assemblies and sealing-assemblies shall be cleared off grit, sand or any other foreign material.
- x) The wheel pin shall be coated with corrosion resistant compound.
- xi) All nuts and bolts shall be tightened.

The other aspects to be inspected and maintained periodically for ensuring proper operation of these gates are as under:

a) Rubber Seals:

Seals shall be inspected for leakages. Locations of excessive leakages shall be recorded for taking remedial measures. Appropriate action to replace the damaged seal needs to be taken immediately after monsoon. Weeping or slight flow in localized area will not require immediate remedial measures. However, measures like tightening of bolts are carried out. Further adjustment is carried out during annual maintenance.

b) Trunnion block assembly and anchorages:

- i) All the nuts and bolts of Trunnion block assembly and its anchorages shall be checked for tightness.
- ii) Check all the welds for soundness and rectify defects.
- iii) Check whether the Yoke girder and thrust block is covered or not. If not, cover it with mild steel plates.
- iv) Cover the trunnion pin with anti- corrosive jelly.
- v) Remove all dirt, grit etc. from trunnion assembly and lubricate trunnion bearings of the gate with suitable water resisting grease as recommended by bearing manufacturers.

c) Gate structures:

- i) Check all the welds for soundness and rectify defects.
- Check welds between arms and horizontal girders as well as between latching bracket and skin plate with the help of magnifying glass for cracks/defects and rectify the defects.
- iii) Clean all drain holes including those in end arms and horizontal girders.
- iv) Check all the nuts and bolts and tighten them. Replace damaged ones.
- v) Check upstream face of skin plate for pitting, scaling and corrosion. Scaling may be filled with weld and grinded. Corroded surface shall be cleaned and painted.

a) Embedded Parts:

- i) All the sill beams and wall plates shall be inspected for crack, pitting etc. and defects shall be rectified.
- ii) The guide roller pins shall be lubricated.

b) General Maintenance:

Defective welding should be chipped out and it should be re-welded duly following the relevant codal provision (IS: 10096, Part-3).

- i) Damaged nuts, bolts, rivets, screws etc. should be replaced.
- ii) Any pitting should be filled up by welding and finished by grinding if necessary.
- iii) The gate leaf, exposed embedded metal parts, hoists and hoist supporting structure etc., should be thoroughly cleaned and repainted when required keeping

in view the original painting system adopted and as per the guidelines contained in IS: 14177.

- iv) Trunnion bearing should be greased as and when required. Keeping trunnion bearings in perfect working condition is very important. All other bolted connections should also be checked up for proper tightness.
- v) Bolts and trunnion bearing housing should be tightened wherever required.
- vi) The seals of the gate should be checked for wear and tear and deterioration. These should be adjusted/replaced as and when necessary.
- vii) The wall plates, sill beams shall be checked and repaired if necessary
- viii) Wire ropes should be properly lubricated.
- ix) Oil level in the worm reduction unit should be maintained by suitable replenishment. Oil seals should also be replaced if required. Lubrication of other parts of hoists such as chains, position indicators and limit switches should also be done.
- x) The stroke of the brake should be reset to compensate for lining wear. Worn out brake linings should be replaced in time.
- xi) Flexible couplings should be adjusted if required.
- xii) Repairs and replacements of all electrical relays and controls should be attended to.
- xiii) Maintenance of alternative sources of Power such as Diesel Generating sets and alternative drives wherever provided should be carried out.
- xiv) The list of essential spare parts to be kept available should be reviewed and updated periodically. The condition of spares should be checked periodically and protective coating given for use. Ensure availability of essential spare parts at site as per the list of essential spares.

4.3.8 Maintenance of Electrically operated fixed hoists

i) General Instructions:

a) Operation of fixed hoist without lifting the gate is not possible and need not therefore be attempted. It will be possible to operate the unit and observe operation of load carrying hoist component when gate is being lifted or lowered. b) Never open any bolt or nut on motor, gear boxes, rope drums and other load carrying hoist components when the gate is in raised position. The gate should be fully closed or rested on the gate latches before carrying out any work on hoist components including motor brake and other electrical equipment.

ii) Inspection and Maintenance

The aspects to be inspected and maintained periodically for ensuring proper operation of Rope drum hoists are as under.

- i. Entrance to all hoist platforms shall be kept locked. All keys shall remain with the shift supervisor.
- ii. A cursory daily inspection shall be made of hoist and gate to ensure that there is no unusual happening.
- iii. Clean all hoisting equipment and hoist platform.
- iv. Check oil level in gearboxes and replenish as and when required with oil of proper grade.
- v. Apply grease of suitable grade by grease gun.
- vi. Lubricate all bearings, bushings, pins, linkages etc.
- vii. Check all the fuses on the power lines.
- viii. All bolts and nuts on gear boxes, hoist drum and shaft couplings should be checked for tightness.
- ix. Check the supply voltage.
- x. Drain sample gear oil from each of the gear boxes. If excessive foreign particles or sludge is found, the gear box shall be drained, flushed and filled with new oil.
- xi. All the geared couplings shall be greased.
- Raise and lower the gate by hoist motor and check for smooth, and trouble free operation of gate without excessive vibration.
- xiii. Observe current drawn by motor at the time of lifting and check if it is more than normal. If so, stop the hoist and investigate the cause and rectify.
- xiv. Check the condition of painting of various components and remove rust wherever noticed and repaint the portion after proper cleaning as per painting schedule.

- xv. All trash, sediments and any other foreign material shall be cleared off the lifting rope and lifting attachment.
- xvi. All ropes shall be checked for wear and tear and if broken wires are noticed, the rope shall be replaced.
- xvii. All the wire ropes shall be checked and all visible oxidation shall be removed.
- xviii. All wire ropes shall be greased with cardium compound.
- xix. Check the overload relays for proper functioning.
- xx. Check all the nuts, bolts, rivets, welds and structural components for hoisting platform and its supporting structure for wear, tear and damage. All damages shall be rectified. All bolts shall be tightened. The portion with damaged painting shall be touched up.
- xxi. Check the pulleys, sheaves and turn-buckles.
- xxii. Raise and lower the gate for its full lift several times (at least three to four) and observe the following:
 - a) Check the limit switches and adjust for design limits.
 - b) The effectiveness and slip of the brakes shall be checked by stopping the gate in raising and lowering operations. The brakes shall be adjusted if needed.
 - c) When the gate is operated, there should not be any noise or chatter in the gears.
- xxiii. Adjust the rope tension of wires if unequal.
- xxiv. Check for all gears and pinions for uneven wear and adjust for proper contact. Grease the gears.
- xxv. Repaint the hoist components, hoisting platform and its supporting structures as per requirement.
- xxvi. The periodic maintenance of commercial equipment like motors, brakes, thrusts etc. shall be carried out as per manufacturers operation and maintenance manual.

4.3.9 Maintenance of Electrical components of Fixed Rope Drum Hoists

- a) The Electrical components to be inspected and maintained periodically are as under;
 - i) Starters should be cleaned free of moisture and dust.
 - ii) Each individual contactor should be tried by hand to make sure that it operates freely.
 - iii) All wearing parts should be examined in order to take note of any wear which may have occurred during operation.
 - iv) If the contactor hums, the contact faces should be cleaned.
 - v) Examine all connections to see that no wires are broken and no connections are loose.
 - vi) Clean the surface of the moving armature and magnet core which comes together when the contactor closes, free of dust or grease of any kind.
 - vii) Examine the mechanical interlocks between the reversing contactor and see when the contact tips of one of the contactor units are touching, it is impossible to get the contact tips of the other unit to touch.
 - viii) The contact tips should be kept free from burns or pits by smoothening with fine sand paper or emery paper.
 - ix) Replace the contact tips which have worn away half-way.
 - x) Do not lubricate the contacts.
 - xi) Blow out windings thoroughly by clean and dry air to clear air passage in the stator and the rotor of any accumulated dirt. The air pressure shall not be too high to damage the insulation.
 - xii) Examine earth connections and motor leads.
 - xiii) Examine motor windings for overheating
 - xiv) Examine control equipment
 - xv) Examine starting equipment for burnt contacts
 - xvi) Check and tighten all nuts and bolts
 - xvii) Clean and tighten all terminals and screw connections all contact surfaces shall be made clean and smooth.
 - xviii) Lubricate the bearings

- xix) Overhaul the controllers
- xx) Inspect and clean circuit breakers.
- xxi) Wipe brush holders and check bedding of brushes.
- xxii) Blow out windings thoroughly by clean and dry air. The pressure shall not be so high that insulation may get damaged.
- xxiii) Check the insulation resistance of the motor between any terminal and the frame. If the measured resistance is less than the prescribed value, then steps shall be taken to dry- out the motors either by passing a low voltage current through the windings or by placing the stator and rotor only in a warm dry place for a day or so.

WARNING: The complete motor shall never be put in an oven

for drying as that may melt the grease out of bearings.

- xxiv) Coat the windings with an approved high temperature resisting insulation enamel or varnish.
- xxv) Over haul the motor, if required.
- xxvi) Check the switch fuse units and renew, if required.
- xxvii) Check resistance or earth connections.

xxviii) Check air gap.

- b) Solenoid Operated Brakes
 - i) All fixing bolts shall be checked and tightened at least once in three months.
 - ii) The magnet stroke should be reset to compensate for wear.
 - Re-adjust the brake when the magnet stroke reaches the value given on the instruction plate.
 - iv) Brake lining should be checked and replaced when required.
 - v) Examine all electrical leads and connections.
 - vi) Rubber bushes or couplings should be checked and replaced if defective.
 - vii) The pins should be tightened.
 - viii) Brake drum shall be cleaned to remove any dust or grease.

4.3.10 Electrical System

Electricity is typically used at a dam for lighting and to operate the gates, hoists, recording equipment, and other miscellaneous equipment. It is important that the Electrical

system be well maintained, including a thorough check of fuses and a test of the system to ensure that all parts are properly functioning. The system should be free from moisture and dirt, and wiring should be checked for corrosion and mineral deposits.

All necessary repairs should be carried out immediately and records of the works kept. Maintain generators used for auxiliary emergency power - change the oil, check the batteries and antifreeze and make sure fuel is readily available.

Monitoring devices usually do not need routine maintenance. Open areas are particularly susceptible to vandalism. As such all electrical fittings like bulbs, lights, loose wires etc. in open areas should be checked routinely and replaced/repaired where needed. The recommendations of the manufacturer should also be referred to.

4.3.11 Maintenance of Metal Gate Components

All exposed, bare ferrous metal of an outlet installation, whether submerged or exposed to air, will tend to rust. To prevent corrosion, exposed ferrous metals must be either appropriately painted (following the paint manufacturer's directions) or heavily greased in respect of moving parts & on surfaces like guides & track seats on which there is movement of gates. When areas are repainted, it should be ensured that paint is not applied to gate seats, wedges, or stems (where they pass through the stem guides), or on other friction surfaces where paint could cause binding. Heavy grease should be applied on friction surfaces to avoid binding. As rust is especially damaging to contact surfaces, existing rust is to be removed before periodic application of grease.

4.3.12 Access Roads

For a dam to be operated and maintained, there must be a safe means of access to it at all times. Access road surfaces must be maintained to allow safe passage of automobiles and any required equipment for servicing the dam in any weather conditions. Routine observations of any cut and fill slopes along the sides of the road should be made. If unstable conditions / slope failure etc. develop, assistance of experienced Engineers/Expert Panels should be arranged and remedial measures initiated. Drains are required to be provided and maintained along roads to remove surface and subsurface drainage. This will prolong the life of the road and help reduce deterioration from rutting. Road surfacing should be repaired or replaced as necessary to maintain the required traffic loadings. In most cases, specialized contractors will be required to perform this maintenance.

4.3.13 General Cleaning

As already suggested, for proper operation of spillways, sluiceways, approach channels, energy dissipation arrangements, discharge conduit, dam slopes, trash racks, debris control devices etc., regular and thorough cleaning and removal of debris is necessary. Cleaning is especially important after large floods, which tend to send more debris into the reservoir. The dam has a horizontal gallery. The dam top road and this gallery are to be cleaned regularly.

4.4 Materials and Establishment Requirements during Monsoon

Materials required during monsoon period for both immediate maintenance and preventive maintenance must be stocked in adequate quantities for emergency situations that may arise.

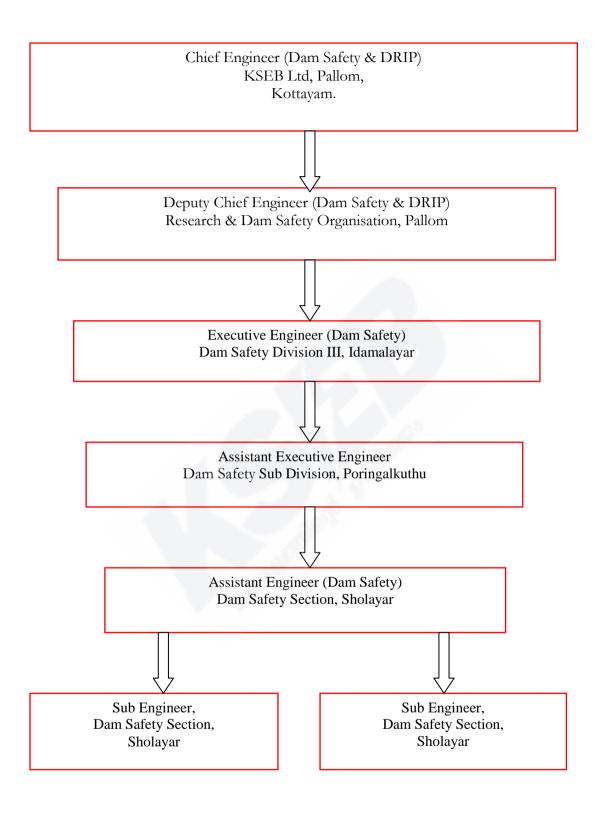
Materials normally required to be stocked in sufficient quantity are:-

- Gunny Bags
- Sand
- Boulders/Wire crates
- Bamboos/Balli's
- Baskets
- Ropes
- Petro max Lamps with Spares
- Torches with spare cells
- Kerosene Oil
- Match Boxes
- Rain Coats

- Gum Boots
- Warning sign indicator
- Danger zone lights

At Sholayar Dam, round the clock patrol is to be carried out during monsoon period. At the same time the man-power requirements during monsoon period are to be enhanced. An Organisation Structure of the Control Unit for normal as well as for monsoon period is given as under:





4.5 Preparation of O&M budget

The O&M budget for Sholayar Dams should essentially include but not be limited to the following items:

- i) Establishment Cost of Regular Staff Salaries and allowances, Bonus, Medical Reimbursement, LTC, Leave Encashment, Pension benefits etc. (As applicable)
- ii) Establishment Cost of Work charged Salaries and allowances, Bonus, Medical Reimbursement, LTC, Leave Encashment, Pension benefits, TA and DA etc. (As applicable)
- iii) Establishment Cost of Daily Wage Staff Salaries and allowances, TA and DA etc. (As applicable)
- iv) Office Expenses Rent for Office, Telephone/Mobile/ any other Telecommunication bills, Electricity bills, Water bills, Office stationaries Day to day office requirements
- v) Motor Vehicles Running and Maintenance cost of inspection vehicles, Cost of hiring of vehicles as required
- vi) Maintenance of Colony Maintenance of staff quarters, colony roads, Electricity, Sanitary and Water supply systems etc.
- vii) T&P -The T&P requirements for offices, colony, works etc. as applicable
- viii) Works Painting, oiling, greasing, overhauling of HM equipment's, Repair/replacement of gates seals & wire ropes, POL for pumps & generator sets, Electricity charges and maintenance of Electric systems of dam site, specific requirements for all Civil, H.M & Electrical maintenance works, Vegetation removal, maintenance/cleaning of drains in dam, maintenance of lift/elevators in dam (as applicable), maintenance of access roads & basic facilities, provision for flood contingency works during monsoon, unforeseen events/items (about 10% of the cost of works) etc.

SL. NO.	BUDGET ITEM	PREVIOUS YEAR COST (Rs)	CURRENT YEAR BUDGET (YR) (Rs)	REMARKS
A. EST	TABLISHMENT	1		
1	SALARY OF REGULAR STAFF			
I	INCLUDING ALL OTHER BENEFITS			
2	TRAVEL EXPENSES			
3	OFFICE EXPENSES			
4	MOTOR VEHICLE EXPENSES			
F	MAINTENANCE OF OFFICE &			
5	COLONY COMPLEX			
	SUB-TOTAL - A			
B. WO	PRKS			
1	CIVIL			
1.1	CONCRETE / MASONRY DAM			
1.2	EARTHEN DAM			
1.3	INTAKE / OUTLETS IN EARTHEN			
1.5	DAMS			
1.4	SLUICES IN CONCRETE / MASONRY			
	DAMS			
1.5	APPROACH / INSPECTION ROADS WITHIN DAM AREA			
2	HYDRO-MECHANICAL			
2.1	SPILLWAY GATES & HOISTS			
	SPILLWAY STOP-LOG & GANTRY			
2.2	CRANE			
	OUTLETS IN EARTHEN DAMS -			
2.3	SERVICE / EMERGENCY GATES &			
	HOISTS			
	SLUICES IN CONCRETE / MASONRY			
2.4	DAMS – SERVICE / EMERGENCY			
	GATES & HOISTS			
3	ELECTRICAL			
3.1	ELECTRICAL FITTINGS, MOTORS,			
5.1	CONTROLS FOR ALL GATE HOISTS			
3.2	POWER SUPPLY LINES			
3.3	ELECTRICAL FITTINGS ON DAM TOP,			
5.5	DAM GALLERIES, ETC			
3.4	STANDBY POWER / DIESEL			
	GENERATOR			
3.5	REMOTE CONTROL/CCTV			

4	INSTRUMENTATION		
5	MISCELLANEOUS WORKS		
6	SALARY OF WORK- CHARGED STAFF INCLUDING ALL BENEFITS		
7	MATERIALS TO BE STORED BEFORE MONSOON		
	SUB-TOTAL - B		
8	CONTINGENCY (10%) ON SUB- TOTAL OF A & B		
9	TOOLS & PLANTS		
	SUB-TOTAL- C		
10	TOTAL ANNUAL COST		

4.6 Maintenance Records

Maintenance records are of utmost importance. A record shall be kept for all maintenance activities, both immediate and preventive maintenance works. Information that must be recorded includes, but not limited to, the following:

- Date and time of maintenance
- Weather conditions
- The type of maintenance
- Name of person or contractor performing maintenance
- Description of work performed
- The length of time it took to complete the work with dates
- Equipment and materials used
- Before and after photographs.

The data should be recorded by the person responsible for maintenance.

Chapter 5 Instrumentation and Monitoring

A dam's instrumentation furnishes data for deciding if the structure is functioning as intended and provides continuous monitoring to warn of any unsafe developments or phenomena that can lead to dam failure by drawing information from a wide spectrum of instruments and procedures, ranging from simple to complex. The program must be based on prevailing geotechnical conditions at the dam and must include consideration of the hydrologic and hydraulic factors present before and after the project is in operation. The extent and nature of the instrumentation depends not only on the complexity of the dam and the size of the reservoir, but also on the potential for threat to life and property losses downstream. The involvement of personnel with experience in the design, installation, regular monitoring, and evaluation of an instrumentation system is of prime importance to the success of the program.

Instruments installed at a dam can indicate occurrence of any anomalous or problematic behavior. They can show that whether the dam behavior is as per design or otherwise. Actual measurements of uplift pressure in a Gravity dam and comparison with the uplift pressure assumed in original designs is an example.

5.1 Instrument Types and Usage

The instruments installed and present status of functioning are appended below.

LIST OF INSTRUMENTATIONS INSTALLED IN SHOLAYAR FLANKING DAM								
Sl. No.	Name of Instruments	Total No installed						
1	V – notch	1						
2	Pressure gauge	1						

5.1.1 A Sholayar Flanking Dam

Table 5.1 Instrumentation present status in Flanking Dam

5.1.2 A Parameters monitored

Uplift Pressure

Pressure gauge is installed in the gallery for measuring uplift pressure.

Water Level

Water level gauge is provided at the left bank of the dam. Daily water levels are taken. During monsoon, hourly readings are taken and recorded.

Seepage Flow

Seepage is measured with V notches

Frequency of Monitoring

Water level is monitored daily and the instruments readings are taken monthly. Spillway details are included in **Chapter 2**.

Seepage assessment

In Sholayar Flanking Dam, there are 20 Periphery Holes and 20 Foundation Drain Holes. Monthly observations of these holes are being carried out. Total seepage is measured using V-notch fitted in the foundation gallery. Seepage details for the years 2016, 2017 and 2018 are tabulated below.

	Reservoir	6.60	
Date	Water Level	V notch reading (cm)	Discharge (Litres/min)
	(m)		
25-01-2016	809.06	3.10	8.282
25-02-2016	805.89	2.60	5.336
23-03-2016	801.90	0	0
29-04-2016	796.08	0	0
23-09-2016	807.72	8.50	103.11
26-10-2016	809.52	6.00	43.165
24-11-2016	809.09	4.40	19.879
23-12-2016	808.85	3.80	13.779

28-01-2017	806.32	2.80	6.422
28-02-2017	802.54	2.00	2.769
31-03-2017	797.36	0	0
29-04-2017	790.96	0	0
24-05-2017	784.56	0	0
09-06-2017	784.71	0	0
25-07-2017	796.59	0	0
22-08-2017	805.01	3.00	7.631
19-09-2017	811.68	6.40	50.723
24-10-2017	810.37	5.00	27.364
28-11-2017	810.80	5.30	31.655
28-12-2017	810.59	4.00	15.664
28-01-2018	808.88	2.80	6.422
23-02-2018	806.17	2.10	3.128
26-03-2018	801.93	0.50	0.087
26-04-2018	796.50	0	0
28-07-2018	811.69	6.00	43.165
28-09-2018	809.00	4.00	15.664
28-10-2018	810.68	4.20	17.696
30-11-2018	809.82	4.10	16.661
07-12-2018	809.73	4.10	16.661

Table 5.2 Seepage Details in Flanking Dam

5.1.1 B Sholayar Main Dam

LIST OF INSTRUMENTATIONS INSTALLED IN SHOLAYAR MAIN DAM						
Sl. No.	No. Name of Instruments Total No instal					
1	V – notch	2				
2 Pressure gauge 1						

Table 5.3 Instrumentation present status in Main Dam

5.1.2 B Parameters monitored

Uplift Pressure

Pressure gauge is installed in the gallery for measuring uplift pressure.

Seepage Flow

Seepage is measured with V notches

Frequency of Monitoring

Instrument readings are taken monthly.

Seepage assessment

In Sholayar Main Dam, there are 90 Periphery Holes and 76 Foundation Drain Holes. Monthly observations of these holes are being carried out. Total seepage is measured using V-notches fitted in the foundation gallery. Seepage details for the years 2016, 2017 and 2018 are tabulated below.

Date	W/ater		readings m)			
Date	(m)	in Block no V	in Block no VII	in Block no V	in Block no VII	Total
25.01.2016	809.06	8.10	7.10	91.404	65.751	157.155
25.02.2016	805.89	6.60	4.90	54.779	26.016	80.795
23.03.2016	801.90	3.20	5.00	8.967	27.364	36.331
29.04.2016	796.08	2.50	3.60	4.837	12.037	16.874
23.09.2016	807.72	15.80	9.00	485.731	118.949	604.680
26.10.2016	809.52	3.20	9.30	8.967	129.110	138.077
24.11.2016	809.09	9.10	8.80	122.280	112.450	234.731
23.12.2016	808.85	8.10	12.10	91.404	249.297	340.701
28.01.2017	806.32	6.60	10.20	54.779	162.650	217.429
28.02.2017	802.54	5.90	9.60	41.389	139.776	181.165
31.03.2017	797.36	4.30	8.40	18.768	100.104	118.872
29.04.2017	790.96	2.30	1.70	3.927	1.844	5.772
24.05.2017	784.56	2.20	1.00	3.514	0.490	4.004

09.06.2017	784.71	2.20	1.40	3.514	1.135	4.649
25.07.2017	796.59	3.10	2.00	8.282	2.769	11.051
22.08.2017	805.01	6.40	7.00	50.723	63.460	114.183
19.09.2017	811.68	11.40	13.30	214.791	315.779	530.569
24.10.2017	810.37	8.50	9.50	103.110	136.164	239.274
28.11.2017	810.80	8.80	9.30	112.450	129.110	241.560
28.12.2017	810.59	8.50	8.00	103.110	88.609	191.719
28.01.2018	808.88	6.10	6.80	44.986	59.024	104.010
23.02.2018	806.17	5.00	5.20	27.364	30.183	57.547
26.03.2018	801.93	3.70	3.90	12.890	14.703	27.593
26.04.2018	796.50	3.00	1.80	7.631	2.128	9.758
28.07.2018	811.68	9.80	11.50	147.170	219.532	366.702
28.09.2018	809.00	7.20	7.50	68.090	75.406	143.496
28.10.2018	810.68	7.00	8.30	63.460	97.151	160.611
30.11.2018	809.82	8.80	8.50	112.450	103.110	215.560
07.12.2018	809.73	8.00	8.20	88.609	94.251	182.861

Table 5.4 Seepage Details in Main Dam

Water Quality

The quality of water including pH value is tested monthly at Regional Analytical laboratory, Ernakulam.

Seismic Activity

There is no Seismic observatory installed at Sholayar Dam.

Weather Conditions

Now the rainfall data are measured with rain gauges. But a full equipped weather station can sense all weather conditions. Automated weather station is needed.

5.2 Data Processing and Evaluation

The monthly reports are prepared for evaluation done.

5.2.1 Data Collection

On daily, fortnightly / monthly basis as the case may be.

5.2.2 Data Presentation

On monthly basis.

5.2.3 Data Interpretation

As per standard practice & on monthly / six monthly / yearly basis or as decided by design authorities.

5.2.4 Data Processing & Evaluation

The steps required to process and evaluate data, whether collected manually or automatically, are the same. Instrument data should be processed and evaluated according to the procedures established by the monitoring program. Accumulation of instrument data by itself does not improve dam safety or protect the public. Interpretation of data, so collected, needs to be carried out judiciously. Help of experienced personnel from the concerned field from Institutes / manufacturers / instrument suppliers could prove to be useful.

5.2.5 Dam Performance Evaluation

Performance evaluation is conducted for safe normal operation involving all concerned engineers / officers before and after monsoon.

In case, the data deviates from expected behavior or design assumptions, action should be taken. The action to be taken depends on the nature of the problem, and should be determined on a case-by-case basis

5.3 Methods of Behavior Prediction

5.3.1 Visual Observations

Observations by on site personnel (dam owners/operators and maintenance personnel) may be the most important and effective means of monitoring the performance

of a dam. An inspector should examine visually walking along the dam alignment for any leakages, any distress, wet spots on d/s face of dam, seepage from foundation gallery etc.

5.3.2 Monitoring Results

Analysis and observation of the instrument readings on water level, leakages, uplift and other parameters can ascertain the visually observed behavior.

Any deviation from the normal behaviour needs to be resolved critically by taking required remedial measures in consultation with senior / experienced engineers.

Со	ompariso	n of seep	oage mea	suremen	ts for dif	ferent ye	ars in Fla	anking d	am
WL (m)	Seepage (lit/min)								
Jan-2014		Jan-2015		Jan-2016		Jan-2017		Jan-2018	
810.55	13.779	811.07	19.879	809.06	8.282	806.32	6.422	808.88	6.422
Apr-2014		Apr-2015		Apr-2016		Apr-2017		Apr-2018	
794.16	0	796.93	0	796.08	0	790.96	0	796.50	0
						3			
Jul-2014		Jul-2015		Jul-2016		Jul-2017		Jul-2018	
799.86	0	792.48	0			796.59	0	811.69	43.165
Oct-2014		Oct-2015		Oct-2016		Oct-2017		Oct-2018	
811.29	21.027	810.10	13.779	809.52	43.165	810.37	27.364	810.68	17.696

	Comparis	son of se	epage me	easurem	ents for d	ifferent	years in N	Main dar	n
WL (m)	Seepage (lit/min)								
		1				1		1	
Jan-2014		Jan-2015		Jan-2016		Jan-2017		Jan-2018	
810.55	121.11	811.07	123.207	809.06	157.155	806.32	217.429	808.88	104.01
Apr-2014		Apr-2015		Apr-2016		Apr-2017		Apr-2018	
794.16	0.455	796.93	34.393	796.08	16.874	790.96	5.772	796.50	9.758

Jul-2014		Jul-2015		Jul-2016		Jul-2017		Jul-2018	
799.86	11.145	792.48	13.804			796.59	11.051	811.68	366.702
Oct-2014		Oct-2015		Oct-2016		Oct-2017		Oct-2018	
811.29	262.754	810.10	185.76	809.52	138.08	810.37	239.274	810.68	160.611

Table 5.5 Comparison of seepage measurements

Chapter 6

REHABILITATION WORKS UNDER DRIP

The rehabilitation works carried out under DRIP include

- 1. Pressure washing, chipping and removing calcinated materials on the downstream face of Sholayar main dam
- 2. Constructing new security cabin at main dam.
- 3. Construction of office building and new security cabin near Sholayar Flanking dam.
- 4. Providing chipping carpet for road to dam and gallery.
- 5. Providing wearing coat on the top roadway of main dam and Flanking dam at Sholayar
- 6. Providing fencing at Sholayar dam premises.
- 7. Providing stone pitching on the downstream face of Sholayar Saddle dam.
- 8. Replacing damaged rubber seals of radial gate of Flanking Dam and Painting the gate.
- 9. Pressure washing, chipping and removing of calcinated materials to the downstream face of Flanking dam.
- 10. Laying power cable from Generator to control gates at Sholayar dam.
- 11. Construction of seepage collecting drain at Sholayar Main dam & Providing steel mesh for gallery outlets to prevent bats at Sholayar dam.
- 12. Painting the parapet, Disperser valve and other connected structures of Main dam.
- 13. Construction of access steps to downstream side of Sholayar Main dam.
- 14. Construction of catwalk way across the spillway gate pillars.
- 15. Replacement of guides of emergency gate at Sholayar main dam.
- 16. Consultancy Services for Geophysical Investigation Studies for the Analysis of Seepage in Sholayar Dam (Main dam and Flanking dam) to Identify the Root Cause of Seepage and to Recommend Remedial Measures to Control Seepage.
- 17. Providing UG cabling systems including fixing of generator, switchboard, distribution board etc. at Sholayar dam and premises.
- 18. Supply and commissioning of high mast light with cost effective lightening system with accessories at Sholayar main dam.

- 19. Supply and commissioning of high mast light with cost effective lightening system with accessories at Sholayar flanking dam.
- 20. Reaming the existing drain holes at Sholayar Main dam & Flanking dam.
- 21. Supply of speed boat at Sholayar dam.
- 22. Providing and fixing retro reflective sign boards.



Fig 6.1 Flanking Dam before construction of catwalk



Fig 6.2 Flanking Dam after construction of catwalk



Fig 6.3 Emergency gate guides before replacement



Fig 6.4 Emergency gate guide after replacement



Fig 6.5 UG cabling systems



Fig 6.6 Retro reflective sign boards



Fig 6.7 D/s face of main dam before pressure washing



Fig 6.8 D/s face of main dam after pressure washing



Fig 6.9 Access steps constructed



Fig 6.10 Office building constructed



Fig 6.11 Security guard room constructed



Fig 6.12 Flanking dam – road work



Fig 6.13 Fencing work

Chapter 7 Updating the Manual

Whenever features of the dam and appurtenant structures change, the O&M Manual must be edited and portions rewritten to reflect these changes. This task is often ignored. Updating information in the O&M Manual should be done whenever major changes like construction of an additional spillway, construction of dam on the upstream etc. take place.

Aspects to be considered when updating include: Increase/decrease in the frequency of an inspection or the maintenance routine based on additional data/ experience acquired, Changes in the operation and/or maintenance procedures based on additional data/experience acquired, Alterations to the project data because of changes/modifications in the dam by way of additional spillway etc.

It is recommended that the O&M Manuals may be reviewed/updated after every 10 years by the respective Dam Owners.